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## **BS476 : Part 21 Fire Resistance Tests**

### **Summary of Data Obtained During Tests on Bolted Beam / Column and Beam / Beam Connections**

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## SUMMARY

### BS476 : PART 21 FIRE RESISTANCE TESTS

### SUMMARY OF DATA OBTAINED DURING TESTS ON BOLTED BEAM / COLUMN AND BEAM / BEAM CONNECTIONS

D.E. Wainman

During the five years 1989-1993 British Steel Technical carried out more than thirty standard fire resistance tests on hot rolled structural steel sections. All the tests were sponsored by the appropriate British Steel Marketing Departments. The range of systems / component configurations investigated in those tests was much wider than in preceding years. Data arising from the tests are being summarised in a series of reports, each one dealing with either a different form of construction or a generic group of test assemblies.

This is the third report issued as part of that series. It contains detailed descriptions of the design, instrumentation and construction for each of twelve bolted beam / column and beam / beam connection assemblies, together with the data arising from them. Data are also included for an indicative column specimen in which the web was encased by blockwork.

Fire resistance periods for all twelve connection assemblies were determined for particular levels of applied loading. It was found that the connection design favourably influenced fire resistance behaviour.

### KEYWORDS

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+ BS 476	Beams
Fire Resistance	Columns
Fire Tests	Load (Mechanical)
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**BS476 : PART 21 FIRE RESISTANCE TESTS****SUMMARY OF DATA OBTAINED DURING TESTS ON BOLTED BEAM / COLUMN AND BEAM / BEAM CONNECTIONS****1. INTRODUCTION**

In 1987 and 1988 research staff based at British Steel Technical, Swinden Laboratories, prepared and published two Compendia<sup>(1),(2)</sup> in which data obtained from standard fire resistance tests were summarised. These documents covered all the British Steel sponsored fire tests carried out in the UK since 1979 in accordance with the requirements of either BS476:Part 8:1972, or the later revision, BS476:Parts 20/21:1987<sup>(3),(4)</sup>. Only tests on hot rolled structural steel sections in which the test members were completely unprotected, or were partially protected by materials used in the fabric of the structure, such as concrete, brick and blockwork, were included. Taking the two documents together, details were given for a total of 62 full scale tests plus a further 31 separate indicative, i.e. unloaded, specimens.

Since the publication of the second compendium a further 40 full scale fire resistance tests have been carried out. The range of systems / component configurations which have been investigated in these tests has been much wider than in the preceding years and has included, for example, tests on:-

- 8 flange plated slim floor beams, (of which 7 were loaded and one was a full length indicative).
- 4 shelf angle floor beams, (of various types).
- 5 composite metal deck floors, (of various types).
- 6 pairs of beam - beam and beam - column connection assemblies.
- 4 composite columns with concrete infill between the flanges.
- 1 column with blockwork infill between the flanges.

Plus, amongst others, three tests on concrete filled circular hollow section columns, two lattice girders formed from square hollow sections, an arched metal deck floor and two fully protected beams. Brief details of all these tests can be found in a recent Technical Note<sup>(5)</sup>. Tests have also been carried out on a number of indicative specimens. These were usually small assemblies which were included in the furnace alongside a full length member, though in some cases they were themselves full scale assemblies.

Much of the data generated from the individual test programmes have already been used extensively by British Steel staff and co-workers in other organisations<sup>(6)</sup>. In particular, they have been used by the Steel Construction Institute<sup>(7-10)</sup>, for the preparation of Design Guides and other documents covering various forms of construction. There is, however, a need to document the test configurations and data in more detail than is usually given in such publications. Having regard to the variety and complexity of the systems examined during the last few years it was considered impractical to attempt to present the data for all of them in one document at the present time. It was therefore decided that a series of reports should be prepared, each one dealing with either a different form of construction or a generic group of test assemblies, and that these reports would eventually be combined to form a third compendium. The first report in the series was issued in September 1993<sup>(11)</sup> and included material relating to the eight flange plated slim floor beams. The second report<sup>(12)</sup>, published in June 1994, presented data relating to the five web encased column assemblies.

This is the third report issued as part of that series. It contains detailed descriptions of the design, instrumentation and construction for each of twelve beam - column or beam - beam connection assemblies, together with the data arising from them which are included in Appendix 1. Also included are data from a

single indicative column specimen in which the web was protected by a blockwork infill between the flanges. The data are presented in a format which is generally consistent with that introduced in the previous compendia. No analyses of the data are included since these have already been incorporated into other publications dealing with design aspects of the various connection types<sup>(13),(14)</sup>. The numerical sequence of the data sheets has been maintained, those in this document being numbered from 112 to 124 inclusive. As in the previous compendia, the thermal data are reduced to summary values at various times throughout the duration of each test. It should be noted, however, that all the thermal and deflection data, usually recorded at one minute intervals, can be made available on PC disks. These may be obtained, on request, from British Steel, Swinden Technology Centre. Further details are given in Appendix 4.

As before, the fire tests reported here form part of an ongoing research programme concerned with the evaluation and prediction of the performance of constructional steelwork in fire. Readers are therefore reminded to exercise caution when using any single test result and not to take it out of context with data for other tests of a similar nature.

## 2. CHANGES TO STANDARDS

The following changes to British Standards have occurred since the publication of the previous compendia.

### 2.1 BS4360:1986 'Weldable Structural Steels'

This standard was withdrawn with effect from March 30th 1990. The parts of BS4360 pertaining to hot rolled sections and plates were replaced from that date by EN 10025 'Hot Rolled Products of Non-Alloy Structural Steels - Technical Delivery Conditions'. BS EN 10025:1990 is the English language version of that standard. The specification requirements for those products and grades not within the scope of EN 10025 were simultaneously re-published unchanged as BS4360:1990. The requirements of the two specifications were compared in an earlier report<sup>(11)</sup>.

As far as the present work is concerned it should be noted that all the tests were carried out prior to March 30th 1990 and hence the BS4360:1986 steel grades have been used throughout this document.

### 2.2 BS476:Parts 20/21:1987

No changes were made to the standard during the period covered by this report. However, discussions are ongoing concerning certain aspects of the standard fire test procedures.

It should be noted that the tests described in this report utilised only the general principles embodied in BS476:Part 20.

## 3. FIRE TESTS ON BOLTED CONNECTION ASSEMBLIES

In this section details are given for twelve fire tests carried out on a range of beam - column and beam - beam connection assemblies. More specifically, eight of the tests were on beam to column flange, (major axis), connections, three were on beam to beam web, (minor axis), connections and one was on a beam to column web, (minor axis), connection. Included within the test programme were four composite beam assemblies, one of which was a shelf angle floor beam. The purpose of the tests was to study the behaviour of three different types of connection under fire conditions, particularly in terms of their moment-rotation characteristics.

At the time of testing no standardised procedure existed for this purpose and so it was only possible to utilise the general principles embodied in BS476:Part 20:1987. All the tests were carried out at the Warrington Fire Research Centre, (WFRC), between December 1988 and March 1990. The major features of the tests are summarised in Tables 1 and 2.

The design and preparation of the twelve assemblies are described individually in Section 3.2. A number of features are, however, common to all of them and these are described in the following section.

### **3.1 Features Common to All of the Test Assemblies**

#### **3.1.1 Steel Quality**

Unless noted otherwise, all of the steel members used in the construction of the test assemblies were manufactured by British Steel and conformed to the requirements of BS4360:1986 Grade 43A. Details of their chemical compositions and mechanical properties are included in the appropriate Data Sheets in Appendix 1.

#### **3.1.2 Dimensions and Section Properties**

The nominal dimensions and section properties, as specified in BS4:Part 1:1980, for the steel members used in the construction of the test assemblies are included in the Data Sheets. The actual dimensions of the members are also given, (where available), together with their calculated section properties.

#### **3.1.3 Fabrication**

The test assemblies were formed from various lengths of universal beam and column sections as described elsewhere in this report, (see Section 3.2). The connections utilised either flush or extended end plates, or double sided web cleats, (see Section 3.1.6). As far as fabrication work was concerned the major requirements were for welding and bolting.

##### **3.1.3.1 Welding**

All welding was by the MMA process using either 3 mm or 4 mm diameter basic coated, hydrogen controlled, general purpose welding rods. The welds used to attach the 20 mm thick column end caps, (used for suspending the assemblies from the loading gantries, see Section 3.1.5), were all full profile continuous 6 mm fillets, (Assemblies 1-8), or 8 mm fillets, (Assemblies 9-12). 6 mm continuous fillets were also used for attaching:-

- the 16 mm thick flush or extended end plates to the stub beams (Assemblies 1, 2, 3, 4, 6, 8, 9, 10 and 12)
- the web stiffeners (Assemblies 1 and 3)
- the seating angles (Assembly No. 7)

In the case of the shelf angle floor beam, (Assembly No. 8), the angles were attached to the beam using 100 mm long intermittent 6 mm fillet welds. 8 mm continuous fillets were used in the fabrication of Assemblies 9-11 for attaching the cross beams to the supporting column members.

Attachment of the 19 mm diameter shear stud connectors used in the construction of the composite beams with metal decking was carried out by specialist contractors using their own equipment.

##### **3.1.3.2 Bolting**

All tests used 20 mm diameter, (M20), Grade 8.8 bolts.

#### **3.1.4 Instrumentation**

##### **3.1.4.1 Temperature Measurement**

The test assemblies were instrumented such that the temperatures attained by the steel sections could be recorded throughout the duration of the heating period. For this purpose 3 mm diameter mineral insulated 'K' type thermocouples, (Ni-Cr/Ni-Al), with insulated hot junctions and Inconel 600 sheaths were used. These thermocouples were embedded to the mid-thickness position of the relevant steel section. Thermocouples were also located along the central longitudinal axis of certain selected bolts used to form

the connections. These thermocouples were only 1.5 mm in diameter but otherwise were identical in type and construction to the larger ones. Each was inserted a distance equal to the bolt head thickness plus the end plate or web cleat thickness so that the hot junction was located at the interface between the connection and the section.

Temperatures were also monitored at various positions within two of the concrete floor slabs. The thermocouples used for these situations were again 'K' type but were usually formed from glass fibre covered Ni-Cr/Ni-Al conductors. The subject is covered in greater detail in Appendix 2.

#### 3.1.4.2 Beam Rotation

Provision was made for monitoring the vertical and horizontal displacements of each of the stub beams throughout each test. The measurements were made using a variety of devices including dial gauges, displacement transducers and inclinometers. The calculated angular rotations of the connections are included in the appropriate Data Sheets in Appendix 1. Three sets of data are presented representing the rotations of the two connections in each test and their mean value. The basic shape of the rotation-time curves is similar for all the tests with the rate of deflection increasing rapidly during the last few minutes of the test.

Deformation of the beams was very small relative to that of the connection at the later stages of the test and could be neglected.

#### 3.1.5 Test Assemblies

The twelve test assemblies were designed to fit within the 4 metre long  $\times$  3 metre wide floor furnace at WFRS. Two tests were carried out in each use of the furnace and this tended to dictate the maximum size of members which could be used. The tests were carried out in two separate series, the first of which comprised eight beam to column, (major axis), assemblies. In the second series the behaviour of three beam to beam, (minor axis), and one beam to column, (minor axis), connections were investigated.

The order in which the assemblies were tested is shown in the table below.

Test Date	WFRS Test No.	Assemblies Tested
07.12.88	44521	No. 2 and No. 4
11.01.89	44519	No. 5 and No. 6
18.01.89	44175	No. 1 and No. 7
25.01.89	44992*	No. 3 and No. 8
27.02.90	46733	No. 9 and No. 10
01.03.90	46736	No. 11 and No. 12

\* Also included an indicative blocked in column specimen, (see Section 4).

The general arrangement of a beam / column test assembly within the WFRS furnace is shown in Fig. 1 and is described in the schedule of components, Table 3. Where reference is made to the left or right hand side of a test assembly this is in accordance with the usual convention in which the direction of viewing is assumed to be from the door end of the test furnace.

##### 3.1.5.1 1st Test Series, (Assemblies 1-8)

The test configuration for all of these was a cruciform comprising two stub beams connected symmetrically to a column as shown in Fig. 1. The beam size used was 305  $\times$  165 mm  $\times$  40 kg/m and the column size was 203  $\times$  203 mm  $\times$  52 kg/m. All the beams supported 120 mm thick concrete or composite floor slabs. The non-composite concrete slabs had movement joints adjacent to the columns to prevent transfer of



moment through the slab. The composite slabs had mesh reinforcement and were continuous i.e. there were no movement joints.

The cruciform specimens were suspended from the underside of the loading gantry by bolting through a plate welded to the top of the 203 × 203 mm column section. Dimensional data for the column top plate are given in Fig. 2. The hydraulic jacks applying the load to the test beams reacted against the gantry and formed a self-equilibrating system. The stub beams were subject to hogging, (negative), moment. The lower part of the column which was in the furnace was unstressed.

### 3.1.5.2 2nd Test Series, (Assemblies 9-12)

The general concepts outlined in the previous section also applied in the second series of tests. However, in order to enable the stub beams to be connected into the web of a beam or column it was necessary to create arrangements of the types shown in Figs. 3(a) and 3(b).

The test configuration for Assemblies 9-11 consisted of a steel section acting as a column with a cross member welded to its lower end. The stub beams framed into and connected with the web of this cross member. In the case of Assembly 12 the stub beams connected directly into the web of the column member. For Assemblies 9-11 the column member was a 305 × 165 mm × 40 kg/m UB whilst for Assembly 12 it was a 254 × 254 mm × 73 kg/m UC. These members were attached to the underside of the loading gantry by means of welded on top plates, the dimensions of which are shown in Fig. 4.

The cross beams used in Assemblies 9-11 were 1 metre long pieces of 406 × 178 mm × 60 kg/m UB. In all four assemblies the stub beams were formed from lengths of 305 × 165 mm × 40 kg/m UB.

The concrete floor slabs in Assemblies 9-11 were terminated approximately 25 mm from the flange tips of the 305 × 165 mm support member and the gap filled with mineral fibre packing. In Assembly 12, 20 mm thick fibre board strips were placed adjacent to the flange tips of the 254 × 254 mm support member to form a movement joint and so prevent transfer of moment through the slab.

### 3.1.6 Connection Types

Three types of connection were selected as being typical of those used in practice. These were:

- Type 1 - a 'rigid' connection using an extended end plate.
- Type 2 - a 'semi-rigid' connection using a flush end plate.
- Type 3 - a 'flexible' connection using double sided web cleats.

#### 3.1.6.1 1st Test Series, (Assemblies 1-8)

Connection types 1 and 2 were tested as unprotected connections, (Assemblies 1, 2 and 3) and connection types 2 and 3 were tested as protected connections with protected beams and columns, (Assemblies 4 and 5). Type 2 and 3 tests were repeated to investigate the influence of composite action, (Assemblies 6 and 7). An additional unprotected test was carried out on a shelf angle floor beam configuration using a type 2 connection, (Assembly No. 8).

In total, four unprotected and four protected connections were tested. The fire protection material used was MANDOLITE P20, a cementitious vermiculite spray. This was applied to a nominal thickness of 20 mm and was intended to provide 60 minutes fire protection, (at a limiting temperature of 550°C), according to reference 15.

In three of the unprotected tests, (Assemblies 1, 2 and 8), the space between the column flanges was filled with light-weight blockwork, (THERMALITE). This was a practical means of providing 30 minutes fire resistance to the columns and was compatible with the 'target' fire resistance of the unprotected beams. In the fourth of these tests, (Assembly No. 3), the column was protected with 50 mm thick VICUCLAD board.

It should be noted that after the first unprotected beam test, (Assembly No. 2), it was found that the beam failed before the connection had suffered any significant deformation. It was speculated that if a beam with a lower section factor had been used then this failure may not have occurred. It was decided that for subsequent unprotected tests of this type, (Assemblies Nos. 1 and 3), the beam would be protected with ceramic fibre blanket but the area in the region of the connection would be left exposed. In this way the connection would be subjected to high temperatures and the beam should not fail prematurely.

### **3.1.6.2 2nd Test Series, (Assemblies 9-12)**

Connection types 2 and 3 were tested as unprotected connections framed into an unprotected beam web, (Assemblies 9 and 11). The type 2 connection test was repeated to investigate the influence of composite action, (Assembly No. 10). A type 2 test was also carried out on an unprotected connection framed into an unprotected column web, (Assembly No. 12).

### **3.1.7 Loading Details**

#### **3.1.7.1 1st Test Series, (Assemblies 1-8)**

Configuration and loading details for all eight tests are summarised in Table 4(a). The plastic moment capacity of the test beams was 171.6 kN m, for a design yield stress of 275 N/mm<sup>2</sup>, according to BS5950:Part 1<sup>(16)</sup>. The mean material yield strength was recorded as 295 N/mm<sup>2</sup>, (the same material was used in the construction of all 8 assemblies).

In Table 4(a) the test moment is expressed relative to the plastic moment capacity of the beam,  $M_p$ , using the nominal yield strength of the steel. This test moment should be reduced by about 7% when expressed in terms of the plastic moment capacity of the beam using the mean measured strength of the steel. The applied moment in the tests was equivalent to approximately two thirds of the moment capacity of the connections. The load applied via the hydraulic jacks was set to a magnitude that would be representative of the shear force that the beam may experience in fire conditions. The position of the jack force was adjusted to give the appropriate moment for the particular connection under investigation.

Summaries of the calculations involved in calculating the hydraulic ram loads are given in Appendix 3.

#### **3.1.7.2 2nd Test Series, (Assemblies 9-12)**

Configuration and loading details for the four tests are summarised in Table 4(b). Similar considerations were applied in this test series as for the earlier one.

### **3.1.8 Failure Criteria**

In the standard fire resistance test, (BS476:Parts 20/21), members in bending are deemed to have failed when their central vertical deflection exceeds SPAN/20, or when the rate of deflection exceeds a specified value for deflections greater than SPAN/30. For most unprotected beams failure occurs rapidly once deflections exceed SPAN/30. However, for protected beams, or special forms of construction such as slim floor and shelf angle floor beams, the rate of increase of deflection is much less rapid.

It follows, therefore, that the maximum rotation at the ends of a simply supported beam is about 9° at the maximum displacement in a fire test. However the end rotation would reduce with increasing moments resisted by the connections. It was thought that in practice connections may be expected to rotate by no more than around 6°, (0.1 radians), maximum under fire conditions. This value was therefore taken to be indicative of the point at which the loads on any particular test assembly should be removed, or reduced.

### **3.1.9 Additional Data**

In some cases heating of a test assembly continued beyond the time at which failure was deemed to have occurred and the loads applied to the stub beams were removed. This was an inevitable consequence of testing two different types of assembly simultaneously. The thermal data recorded during these situations

have not been included in the Data Sheets but can be made available on request. In some tests it was possible to reduce the test load to a lower magnitude once failure was imminent thereby giving a measure of the performance of the connection under reduced moment.

### 3.2 Loaded Test Assemblies

The following sections describe in greater detail aspects concerning the construction, instrumentation and loading of the twelve test assemblies.

#### 3.2.1 Assembly No. 1

A non-composite construction consisting of a universal column section of serial size  $203 \times 203 \text{ mm} \times 52 \text{ kg/m} \times 1985 \text{ mm}$  long and two universal beams of serial size  $305 \times 165 \text{ mm} \times 40 \text{ kg/m} \times 1280 \text{ mm}$  long. The stub beams were attached to the column flanges by means of 16 mm thick extended end plate connections using M20 Grade 8.8 bolts. Dimensional data for the end plates are given in Fig. 5(a). Constructional details for the assembly are shown in Fig. 6. 175 mm wide  $\times$  60 mm deep  $\times$  6 mm thick web stiffeners were welded between the column flanges at positions which corresponded with the lower flanges of the stub beams. A 120 mm thick  $\times$  620 mm wide Grade 25 normal weight concrete floor slab was cast over the stub beams. No expansion joints were provided in the concrete but a 20 mm thick fibre-board insert was included adjacent to the column, (see Section 3.1.5.1).

The web of the column extending into the furnace below the concrete floor slab was protected by 'THERMALITE' blockwork cemented into the flange / web cavities. The blockwork rested on an 8 mm thick plate welded to the bottom end of the column. The blocks were cemented into the section cavities on both sides and were finished flush with, or slightly proud of, the flange tips. A nominal 10 mm thick mortar joint was maintained between adjacent blocks and the block / steel interfaces. The blockwork was cut to fit around the web stiffeners. The stub beams were protected by 12 mm thick ceramic fibre blanket which extended to within 40 mm of the end plates on the flanges and 100 mm on the web. The protection was applied in the conventional manner using shot fired pins and washers. The actual connections remained unprotected.

A total of 18 thermocouples were used to monitor the temperature of the steel sections throughout the test with a further six located in selected exposed and unexposed bolts. Six thermocouples were located in the stub beams adjacent to the connections, (25 mm from the ends of the beams), and six remote from the connections, (500 mm from the ends of the beams). The thermocouples in the upper and lower flanges were located at the quarter width positions. Thermocouples were also located in the end plates and in the column extending into the furnace. All the thermocouple positions are indicated in Figs. 7 and 8.

Loads of 6.9 tonnes were applied to each of the beams at a distance of 980 mm from the outside face of the column flange. The loads were maintained for 45 minutes after which time they were reduced to 5.1 tonnes. The loads were removed completely after 49 minutes. Heating of the assembly continued until 120 minutes had elapsed, (tested alongside Assembly No. 7).

Data for this test are summarised in Data Sheet No. 112.

#### 3.2.2 Assembly No. 2

A non-composite construction consisting of the same size universal beam and column sections as given for Assembly No. 1. The stub beams were attached to the column flanges by means of flush end plate connections using M20 Grade 8.8 bolts. Dimensional data for the end plates are given in Fig. 5(b). Details of the assembly are shown in Fig. 9. A 120 mm thick  $\times$  620 mm wide Grade 25 normal weight concrete floor slab was cast over the stub beams, (details as for Assembly No. 1).

The web of the column extending into the furnace below the concrete floor slab was protected with 'THERMALITE' blockwork as described for Assembly No. 1. No protection was applied to the beams which remained exposed to the heating conditions of the furnace throughout the test. The thermocouple positions were as described for Assembly No. 1 except that since a flush end plate connection was used only

four bolts were monitored. An additional six thermocouples were included on this test for the purpose of obtaining a vertical thermal profile in the web of one of the beams. The additional thermocouples were located at 10, 30 and 50 mm from both the upper and lower beam flanges and at 25 mm from the end of the beam adjacent to the connection, (see Fig. 10).

Loads of 6.7 tonnes were applied to each of the beams at a distance of 490 mm from the outside face of the column flange. The loads were maintained for a period of 29 minutes and were then removed. Heating of the assembly continued until 122 minutes had elapsed, (tested alongside Assembly No. 4).

Data for this test are summarised in Data Sheet No. 113.

### 3.2.3 Assembly No. 3

This assembly was similar in terms of its construction to Assembly No. 1, i.e. non-composite, extended end plate connections, web stiffeners and normal weight concrete floor slab. The 600 mm of column extending into the furnace below the stub beams was protected, (boxed in), using 50 mm thick 'VICUCLAD' board. The stub beams were protected with ceramic fibre blanket as described for Assembly No. 1. The connections remained unprotected. Construction details for the assembly are shown in Fig. 11. The thermocouple positions were as described for Assembly No. 1.

Loads of 6.7 tonnes were applied to each of the beams at a distance of 490 mm from the outside face of the column flange. The loads were maintained for 97 minutes and then removed. The test was terminated after 98 minutes.

Data for this test are summarised in Data Sheet No. 114.

### 3.2.4 Assembly No. 4

This assembly was similar in terms of its construction to Assembly No. 2, i.e. non-composite, flush end plate connections and normal weight concrete floor slab. There was no blockwork infill between the column flanges on this assembly. The area of the assembly exposed to the heating conditions of the test was protected with spray applied 'MANDOLITE P20'. The coating thickness was nominally 20 mm and had a nominal density of 320 kg/m<sup>3</sup>. Constructional details for the assembly are shown in Fig. 12. The material had been applied on 10th October 1988 and therefore had an age of 58 days at the time of testing. The moisture content measured on the day of the test was 1.7% w/w.

The thermocouple arrangement was the normal one of 18 in the steelwork plus 4 in selected bolt positions, (flush end plate). Loads of 6.7 tonnes were applied to each of the beams at a distance of 490 mm from the outside face of the column flange. The loads were maintained for a period of 122 minutes at which time they were removed and the test terminated.

Data for this test are summarised in Data Sheet No. 115.

### 3.2.5 Assembly No. 5

A non-composite construction consisting of the same size universal beam and column sections as given for Assembly No. 1. The stub beams were attached to the column flanges by means of double sided web cleat connections using M20 Grade 8.8 bolts. The cleats were formed from lengths of 90 × 90 mm × 8 mm thick rolled steel angle. Constructional data for the cleated connections are given in Figs. 13 and 14. A 120 mm thick × 620 mm wide Grade 25 normal weight concrete floor slab was cast over the stub beams, (details as for Assembly No. 1).

The area of the assembly exposed to the heating conditions of the test was protected with spray applied 'MANDOLITE P20', (details as for Assembly No. 4). The age of the material at the time of testing was 93 days. The moisture content measured on the day of the test was 1.6% w/w. The measured thickness of the protection was as shown in the table below.

Side	Position	Mean Thickness (mm)
Left	Beam Web	19.8
Right	Beam Web	21.0
Left	Lower Flange	20.25
Right	Lower Flange	20.5
	Column Web	20.5

The thermocouple arrangement was the normal one of 18 in the steelwork plus 6 in selected bolt positions, (see Fig. 15). Loads of 3.3 tonnes were applied to each of the beams at a distance of 490 mm from the outside face of the column flange. The loads were maintained for a period of 115 minutes and were then removed. Heating of the assembly continued until 128 minutes had elapsed, (tested alongside Assembly No. 6).

Data for this test are summarised in Data Sheet No. 116.

### 3.2.6 Assembly No. 6

A composite construction consisting of the same size universal beam and column sections as given for Assembly No. 1. The stub beams were attached to the column flanges by means of flush end plate connections, (see Assembly No. 2). Richard Lees 'HOLORIB' steel decking was fixed to the upper beam flanges using 19 mm diameter  $\times$  95 mm long shear stud connectors. These were situated at 300 mm centres in a single line over the web of the section. A 120 mm thick  $\times$  1000 mm wide light-weight, (LYTAG), Grade 25 concrete floor slab was cast over the decking. An A142 anti-crack mesh was incorporated into the concrete slab. No expansion joints were provided and there were no movement joints adjacent to the column. Details of the construction are shown in Fig. 16.

The area of the assembly exposed to the heating conditions of the test was protected with spray applied 'MANDOLITE P20', (details as for Assembly No. 4). The age of the material at the time of testing was 93 days. The moisture content measured on the day of the test was 1.6% w/w. The measured thickness of the protection was as shown in the table below.

Side	Position	Mean Thickness (mm)
Left	Beam Web	19.6
Right	Beam Web	19.8
Left	Lower Flange	20.25
Right	Lower Flange	21.0
	Column Web	20.25

The thermocouple arrangement was the normal one of 18 in the steelwork plus 4 in selected bolt positions. Loads of 6.7 tonnes were applied to each of the beams at a distance of 980 mm from the outside face of the column flange. The loads were maintained for 100 minutes after which time they were reduced to 5.0 tonnes. The loads were removed completely after 123 minutes. The test was terminated after 128 minutes.

Thermocouples were also placed at various positions within the concrete floor slab, see Appendix 2.

Data for this test are summarised in Data Sheet No. 117.

### 3.2.7 Assembly No. 7

A composite construction consisting of the same size universal beam and column sections as given for Assembly No. 1. The stub beams were attached to the column flanges by means of double sided web cleat connections, (see Assembly No. 5). An additional feature of this assembly was the inclusion of two seating angles situated immediately below the stub beams. Details are given in Figs. 13 and 14. These were welded to the column flanges using continuous 6 mm fillets. Richard Lees 'HOLORIB' steel decking, 19 mm diameter shear connectors, A142 mesh and light-weight concrete were used to form the floor slab, (see Assembly No. 6).

The area of the assembly exposed to the heating conditions of the test was protected with spray applied 'MANDOLITE P20', (details as for Assembly No. 4). The age of the material at the time of testing was 100 days. The moisture content measured on the day of the test was 1.6% w/w.

The thermocouple arrangement was the normal one of 18 in the steelwork plus 6 in selected bolt positions, (see Fig. 15). Loads of 6.6 tonnes were applied to each of the beams at a distance of 490 mm from the outside face of the column flange. The load was maintained for 111 minutes and then removed. Heating of the assembly continued for a further 9 minutes.

Thermocouples were also placed at various positions within the concrete floor slab, see Appendix 2.

Data for this test are summarised in Data Sheet No. 118.

### 3.2.8 Assembly No. 8

A composite shelf angle floor beam construction using the same size universal beam and column sections as given for Assembly No. 1. The stub beams were attached to the column flanges by means of flush end plate connections, (see Assembly No. 2). The shelf angles were formed from lengths of 125 × 75 mm × 12 mm thick rolled steel angle, (Grade 43A). These were attached to the webs of the stub beams using 100 mm long intermittent 6 mm fillet welds along both the top and bottom edges. The 75 mm vertical leg of the angle pointed upwards. Dimensional details relating to the location of the shelf angles are given in Fig. 17.

Richard Lees, 'SUPER HOLORIB' steel decking was fixed to the angles using 19 mm diameter x 95 mm long shear connectors and in-situ light-weight, (LYTAG), Grade 25 concrete was used to form the floor slab. The shear connectors were spaced at 300 mm centres in a single line over the web of the section. The finished floor level was 25 mm above the upper flange of the beams. An A142 mesh was incorporated into the concrete slab. No expansion joints were provided and there were no movement joints adjacent to the column. The overall dimensions of the floor slab were 120 mm thick × 1000 mm wide.

The web of the column extending into the furnace below the concrete floor was protected with 'THERMALITE' blockwork as described for Assembly No. 1. No protection was applied to the lower part of the beams which remained exposed to the heating conditions of the furnace throughout the test.

A total of 30 thermocouples were used to monitor the temperature of the steel sections with a further four located in the exposed and unexposed bolts. All the thermocouple positions are indicated in Figs. 18 and 19.

Loads of 6.6 tonnes were applied to each of the beams at a distance of 490 mm from the outside face of the column flange. The loads were maintained for 70 minutes after which time they were reduced to 3.0 tonnes. The loads were removed completely after 90 minutes. Heating of the assembly continued until 98 minutes had elapsed, (tested alongside Assembly No. 3).

Data for this test are summarised in Data Sheet No. 119.

### 3.2.9 Assembly No. 9

A non-composite beam - beam (minor axis) connection consisting of:

- Item 1 - A universal beam section of serial size 305 × 165 mm × 40 kg/m × 1080 mm long which was used as a column.
- Item 2 - A universal beam section of serial size 406 × 178 mm × 60 kg/m × 1000 mm long which was welded to the bottom of item 1.
- Item 3 - Two universal beam sections of serial size 305 × 165 mm × 40 kg/m × 1380 mm long. Each of these stub beams had a 20 mm thick flush end plate welded to one end.

The end plates were connected by M20 Grade 8.8 bolts which passed through the web of the 406 × 178 mm beam, (item 2). Dimensional data for the end plates are given in Fig. 20. Constructional details for the assembly are shown in Fig. 21.

A 100 mm thick × 600 mm wide Grade 25 normal weight concrete floor slab was cast over the stub beams. The whole of the exposed steelwork was protected by 25 mm thick ceramic fibre blanket, (nominal density 96 kg/m<sup>3</sup>), which extended to within 40 mm of the end plates on the flanges and 100 mm on the web. The protection was applied in the conventional manner using shot fired steel pins and washers.

A total of 15 thermocouples were used to monitor the temperature of the steel sections throughout the test with a further four located in the bolts. The arrangement of the thermocouples was generally similar to that adopted for the beam to column tests, (Assemblies 1-8). All the thermocouple positions are indicated in Figs. 22 and 23. It should be noted that positions F2 and F3 are on the cross beam and not the test beams.

Loads of 5.7 tonnes were applied to each of the beams at a distance of 590 mm from the outside face of the column web. The loads were maintained for 46 minutes and then removed. The test was terminated after 47 minutes, (tested alongside Assembly No. 10).

Data for this test are summarised in Data Sheet No. 120.

### 3.2.10 Assembly No. 10

A composite beam - beam (minor axis) connection consisting of the same size of universal beam and column sections as given for Assembly No. 9. Fabrication details for the steelwork were identical to Assembly No. 9. The stub beams supported a 1000 mm wide Richard Lees 'RIBDECK 60' steel deck onto which a 120 mm thick Grade 25 normal weight concrete floor slab was cast. An A142 anti-crack mesh was incorporated into the concrete slab. Constructional details are given in Fig. 24.

The whole of the exposed steelwork, with the exception of the small regions immediately adjacent to the connections, was protected with ceramic fibre blanket as described for Assembly No. 9. The thermocouple arrangement was as described for Assembly No. 9. Loads of 6.2 tonnes were applied to each of the beams at a distance of 1080 mm from the outside face of the column web. The loads were maintained for a period of 47 minutes at which time they were removed and the test terminated.

Data for this test are summarised in Data Sheet No. 121.

### 3.2.11 Assembly No. 11

A non-composite beam - beam (minor axis) connection consisting of the same size of universal beam and column sections as given for Assembly No. 9. The stub beams were attached to the web of the 406 × 178 mm beam, (item 2), by means of double sided web cleat connections using M20 Grade 8.8 bolts. The

cleats were formed from lengths of  $90 \times 90 \text{ mm} \times 8 \text{ mm}$  thick rolled steel angle. Constructional data for the cleated connections are given in Figs. 25 and 26.

A 100 mm thick  $\times$  600 mm wide Grade 25 normal weight concrete floor slab was cast over the stub beams. The whole of the exposed steelwork, with the exception of the small regions immediately adjacent to the connections was protected with ceramic fibre blanket as described for Assembly No. 9. The thermocouple arrangement was the normal one of 15 in the steelwork, (see Fig. 22), plus 8 in selected bolt positions, (see Fig. 26).

Loads of 2.7 tonnes were applied to each of the beams at a distance of 590 mm from the outside face of the column web. The loads were maintained for a period of 30 minutes at which time they were removed. Heating of the assembly continued until 72 minutes had elapsed, (tested alongside Assembly No. 12).

Data for this test are summarised in Data Sheet No. 122.

### 3.2.12 Assembly No. 12

A non-composite beam - column (minor axis) connection consisting of a 1985 mm long universal column section of serial size  $254 \times 254 \text{ mm} \times 73 \text{ kg/m}$  and two 1380 mm long universal beams of serial size  $305 \times 165 \text{ mm} \times 40 \text{ kg/m}$ . Each of the stub beams had a 20 mm thick flush end plate welded to them and these were connected by M20 Grade 8.8 bolts which passed through the web of the  $254 \times 254 \text{ mm}$  column. Dimensional data for the end plates are given in Fig. 5(b). The arrangement is shown schematically in Fig. 3(b).

A 100 mm thick  $\times$  600 mm wide Grade 25 normal weight concrete floor slab was cast over the stub beams. No expansion joints were provided but a 20 mm thick fibre-board insert was included adjacent to the column. The whole of the exposed steelwork, with the exception of the small regions immediately adjacent to the connections was protected with ceramic fibre blanket as described for Assembly No. 9.

A total of 16 thermocouples were used to monitor the temperature of the steel sections throughout the test with a further four located in the bolts. The arrangement of the thermocouples was generally similar to that adopted for the beam to column tests, (Assemblies 1-8). All the thermocouple positions are indicated in Figs. 27 and 28.

Loads of 5.7 tonnes were applied to each of the beams at a distance of 590 mm from the outside face of the column web. The loads were maintained for a period of 69 minutes and then removed. The test was terminated after 72 minutes.

Data for this test are summarised in Data Sheet No. 123.

## 4. INDICATIVE TEST ASSEMBLY

During the course of the foregoing test programme the opportunity arose to include a short, (1 m), length of column section in which the web was encased in blockwork. The thermal data recorded, together with other relevant information are summarised in Data Sheet No. 124.

The test assembly consisted of a universal column section of serial size  $356 \times 406 \text{ mm} \times 634 \text{ kg/m}$  which was partially protected by blockwork cemented into the flange / web cavities. The protection consisted of 'CELCON' aerated, autoclaved concrete blocks, each nominally 440 mm long  $\times$  210 mm deep  $\times$  150 mm thick, which were cemented into the section cavities and finished flush with, or slightly proud of, the flange tips. A nominal 10 mm thick mortar joint was maintained between adjacent blocks and the block / steel interfaces.

The following properties were quoted by the manufacturer for the 'CELCON' blocks.

- Typical stabilised water content 3%
- Density (at 3% moisture level) 680 kg/m<sup>3</sup>



●	Density (fully dried)	650 kg/m <sup>3</sup>
●	Compressive Strength	4.0 N/mm <sup>2</sup>
●	Coefficient of expansion	$8 \times 10^{-6}$ mm/°C

Seven thermocouples were used to monitor the temperature of the steel section throughout the 98 minute heating period of the test. The thermocouples, which were at the mid-height of the section, were positioned as shown in Fig. 29.

The assembly was tested on 25th January 1989 as part of WFRC 44992. It was positioned vertically in the furnace and was raised approximately 300 mm from the furnace floor on a concrete plinth.

## 5. CONCLUSIONS

Data arising from twelve fire resistance tests, involving a range of beam / column and beam / beam connection configurations, have been collated and reported. No standardised test procedures existed for the purpose of determining the moment rotation and load carrying characteristics of different types of connections under fire conditions. However, all the procedures employed the general principles and heating conditions described in BS476:Part 20:1987.

Details of the test assemblies are given together with summaries of the material properties, loading calculations and the thermal data recorded. Similar information is also included for an indicative blocked in column assembly.

Fire resistance periods for all twelve connection assemblies were determined for particular levels of applied loading. It was found that the connection design favourably influenced fire resistance behaviour. It should be noted that this aspect has now been incorporated into structural design procedures for the fire limit state.

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**TABLE 1**  
**SUMMARY OF THE MAJOR FEATURES OF THE CONNECTION TEST ASSEMBLIES**  
**1ST SERIES - BEAM TO COLUMN MAJOR AXIS TESTS**

Test Assembly No.	Test Order	Test Date	WFRC Test No.	Construction Details	Fire Resistance Rating	Data Sheet No.
1	3	18.01.89	44175	Extended end plate connections. Web stiffeners between column flanges. Non-composite normal weight concrete floor slab. Column web protected by Thermalite blockwork infill. Beams protected by 12 mm thick ceramic fibre blanket. Connections unprotected.	45 min. at 0.4 M <sub>p</sub> Continued to: 49 min. at 0.3 M <sub>p</sub>	112
2	1	07.12.88	44521	Flush end plate connections. Non-composite normal weight concrete floor slab. Column web protected by Thermalite blockwork infill Beams and connections both unprotected.	29 min. at 0.2 M <sub>p</sub>	113
3	4	25.01.89	44992	Extended end plate connections. Web stiffeners between column flanges. Non-composite normal weight concrete floor slab. Column protected by 50 mm thick Vicuclad board. Beams protected by 12 mm thick ceramic fibre blanket. Connections unprotected.	97 min. at 0.2 M <sub>p</sub>	114
4	1	07.12.88	44521	Flush end plate connections. Non-composite normal weight concrete floor slab. Beams, column and connections protected by 20 mm thick spray applied Mandolite P20.	122 min. at 0.2 M <sub>p</sub>	115

(Cont ...)

**TABLE 1**  
**SUMMARY OF THE MAJOR FEATURES OF THE CONNECTION TEST ASSEMBLIES**  
**1ST SERIES - BEAM TO COLUMN MAJOR AXIS TESTS (Continued)**

Test Assembly No.	Test Order	Test Date	WFRC Test No.	Construction Details	Fire Resistance Rating	Data Sheet No.
5	2	11.01.89	44519	Double sided web cleat connections. Non-composite normal weight concrete floor slab. Beams, column and connections protected by 20 mm thick spray applied Mandolite P20.	115 min. at 0.1 M <sub>p</sub>	116
6	2	11.01.89	44519	Flush end plate connections. Composite light-weight concrete floor slab. 'Holorib' decking and shear studs. Shear studs 19 mm dia. x 95 mm long at 300 mm centres. A142 mesh in floor slab. Beams, column and connections protected by 20 mm thick spray applied Mandolite P20.	100 min. at 0.4 M <sub>p</sub> Continued to: 123 min. at 0.3 M <sub>p</sub>	117
7	3	18.01.89	44175	Double sided web cleat connections. Seating cleats below test beams. Composite light-weight concrete floor slab. 'Holorib' decking and shear studs. Shear studs 19 mm dia. x 95 mm long at 300 mm centres. A142 mesh in floor slab. Beams, column and connections protected by 20 mm thick spray applied Mandolite P20.	111 min. at 0.2 M <sub>p</sub>	118
8	4	25.01.89	44992	Shelf angle floor beam. Flush end plate connections. Composite light-weight concrete floor slab. 'Super Holorib' decking and shear studs. Shear studs 19 mm dia. x 95 mm long at 300 mm centres. A142 mesh in floor slab. Column web protected by Thermalite blockwork infill. Beams and connections both unprotected.	70 min. at 0.2 M <sub>p</sub> Continued to: 90 min. at 0.1 M <sub>p</sub>	119

**TABLE 2**  
**SUMMARY OF THE MAJOR FEATURES OF THE CONNECTION TEST ASSEMBLIES**  
**2ND SERIES - BEAM TO BEAM AND COLUMN MINOR AXIS TESTS**

Test Assembly No.	Test Order	Test Date	WFRC Test No.	Construction Details	Fire Resistance Rating	Data Sheet No.
9	1	27.02.90	46733	Flush end plate connections. Non-composite normal weight concrete floor slab. Steel sections protected by 25 mm thick ceramic fibre blanket. Connections unprotected.	46 min. at 0.2 Mp	120
10	1	27.02.90	46733	Flush end plate connections. Composite normal weight concrete floor slab. 'Ribdeck 60' decking. A142 mesh in floor slab. Steel sections protected by 25 mm thick ceramic fibre blanket. Connections unprotected.	47 min. at 0.4 Mp	121
11	2	01.03.90	46736	Double sided web cleat connections. Non-composite normal weight concrete floor slab. Steel sections protected by 25 mm thick ceramic fibre blanket. Connections unprotected.	30 min. at 0.1 Mp	122
12	2	01.03.90	46736	Flush end plate connections. Non-composite normal weight concrete floor slab. Steel sections protected by 25 mm thick ceramic fibre blanket. Connections unprotected.	69 min. at 0.2 Mp	123

**TABLE 3**  
**SCHEDULE OF COMPONENTS SHOWN IN FIG. 1**

Reference No.	Description
1	Furnace lining
2	Loading gantry
3	Hydraulic rams
4	203 × 203 mm × 52 kg/m UC × 1985 mm long
5	305 × 165 mm × 40 kg/m UB × 1280 mm long
6	Connection under test
7	620 mm wide × 120 mm thick concrete floor slab
8	Blockwork perimeter wall
9	Flexible seal to beam end. (Mineral fibre.)

**TABLE 4(a-b)**  
**SUMMARY OF LOADING CONDITIONS**

**(a) Test Assemblies 1-8**

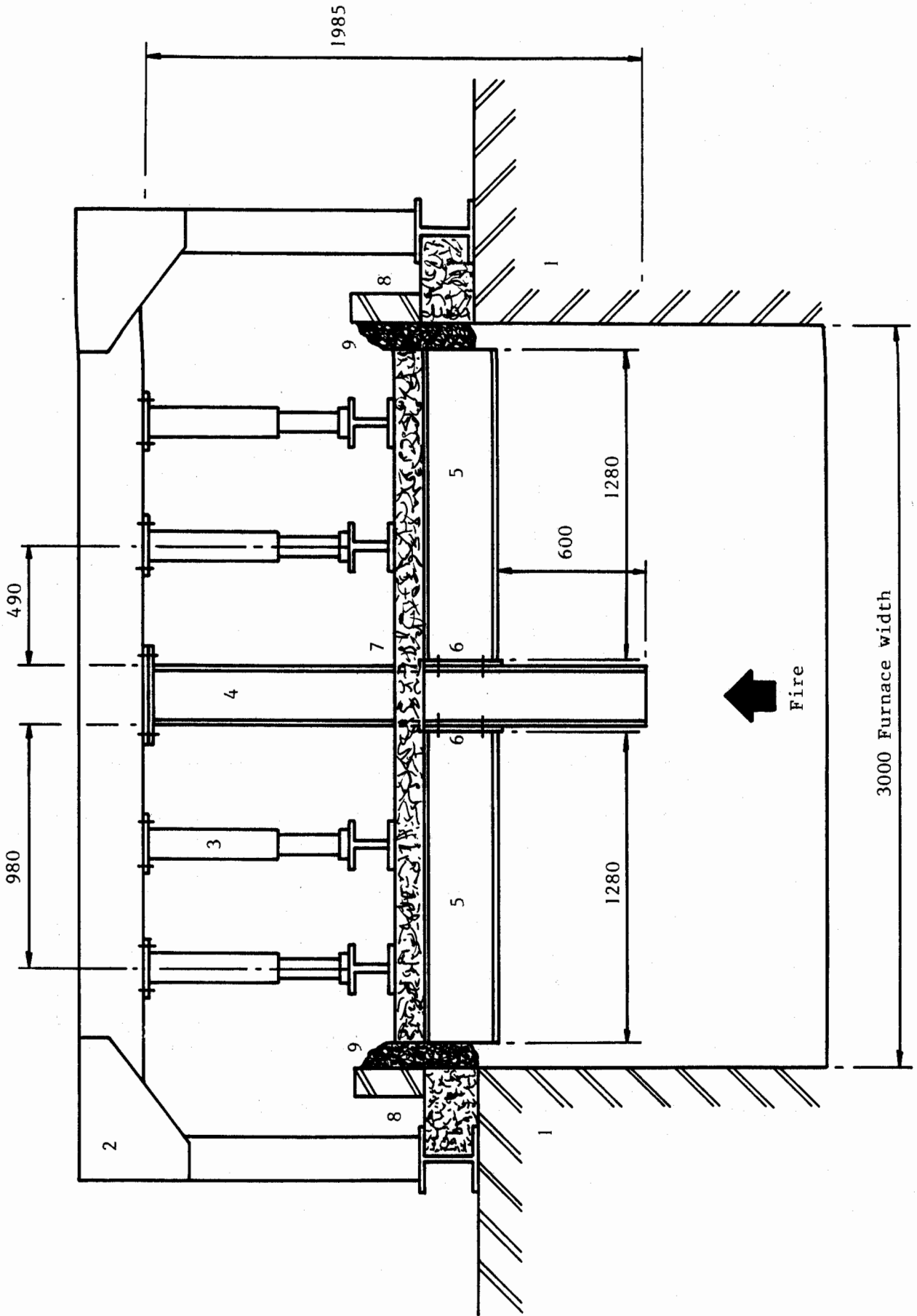
Test Assembly No.	Applied Load (tonnes)	Lever Arm (mm)	Moment * Capacity of Connection Assembly	Test Moment	Fire Resistance Period (min)
1	6.9	980	0.6 M <sub>p</sub>	0.4 M <sub>p</sub>	45
	5.1			0.3 M <sub>p</sub>	49
2	6.7	490	0.3 M <sub>p</sub>	0.2 M <sub>p</sub>	29
3	6.7	490	0.6 M <sub>p</sub>	0.2 M <sub>p</sub>	97
4	6.7	490	0.3 M <sub>p</sub>	0.2 M <sub>p</sub>	122
5	3.3	490	0.1 M <sub>p</sub>	0.1 M <sub>p</sub>	115
6	6.7	980	0.45 M <sub>p</sub>	0.4 M <sub>p</sub>	100
	5.0			0.3 M <sub>p</sub>	123
7	6.6	490	0.25 M <sub>p</sub>	0.2 M <sub>p</sub>	111
8	6.6	490	0.42 M <sub>p</sub>	0.2 M <sub>p</sub>	70
	3.3			0.1 M <sub>p</sub>	90

**(b) Test Assemblies 9-12**

Test Assembly No.	Applied Load (tonnes)	Lever Arm (mm)	Moment * Capacity of Connection Assembly	Test Moment	Fire Resistance Period (min)
9	5.7	590	0.3 M <sub>p</sub>	0.2 M <sub>p</sub>	46
10	6.2	1080	0.45 M <sub>p</sub>	0.4 M <sub>p</sub>	47
11	2.7	590	0.1 M <sub>p</sub>	0.1 M <sub>p</sub>	30
12	5.7	590	0.3 M <sub>p</sub>	0.2 M <sub>p</sub>	69

Notes:- M<sub>p</sub> is the plastic moment capacity of the steel beam = 171.6 kN m  
(for a nominal yield stress of 275 N/mm<sup>2</sup>)

\* As given in Refs. 13 and 14

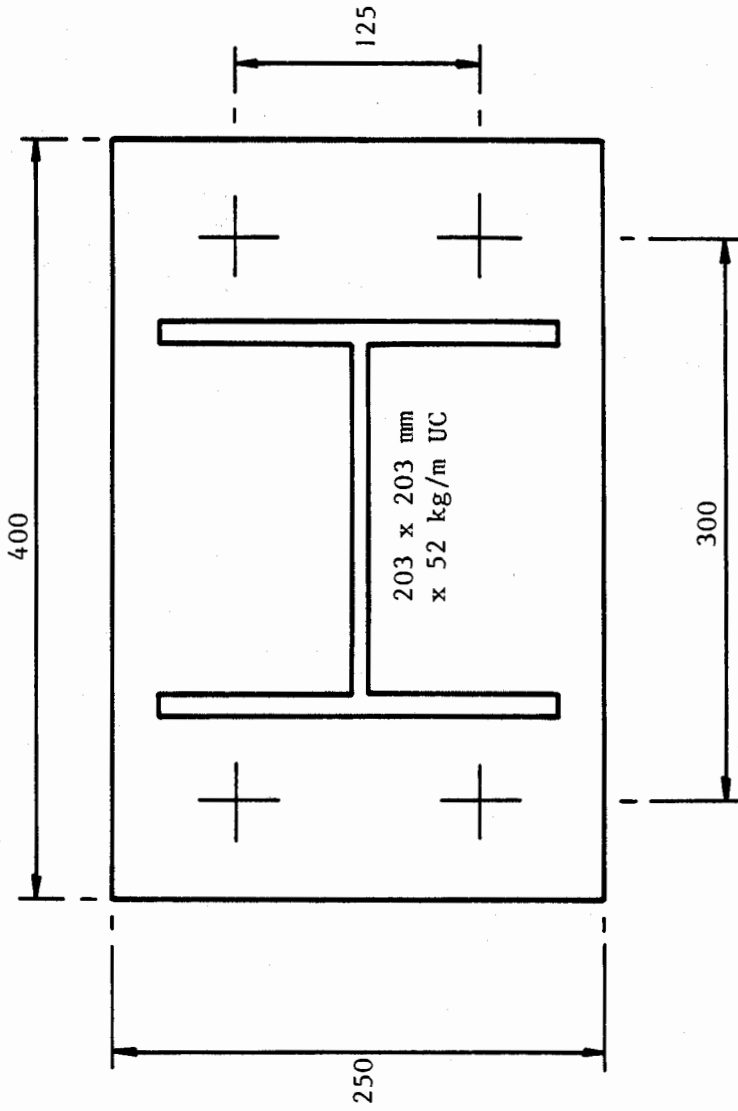


(R4/3640)

GENERAL ARRANGEMENT OF COMPONENTS FOR BEAM/COLUMN TESTS

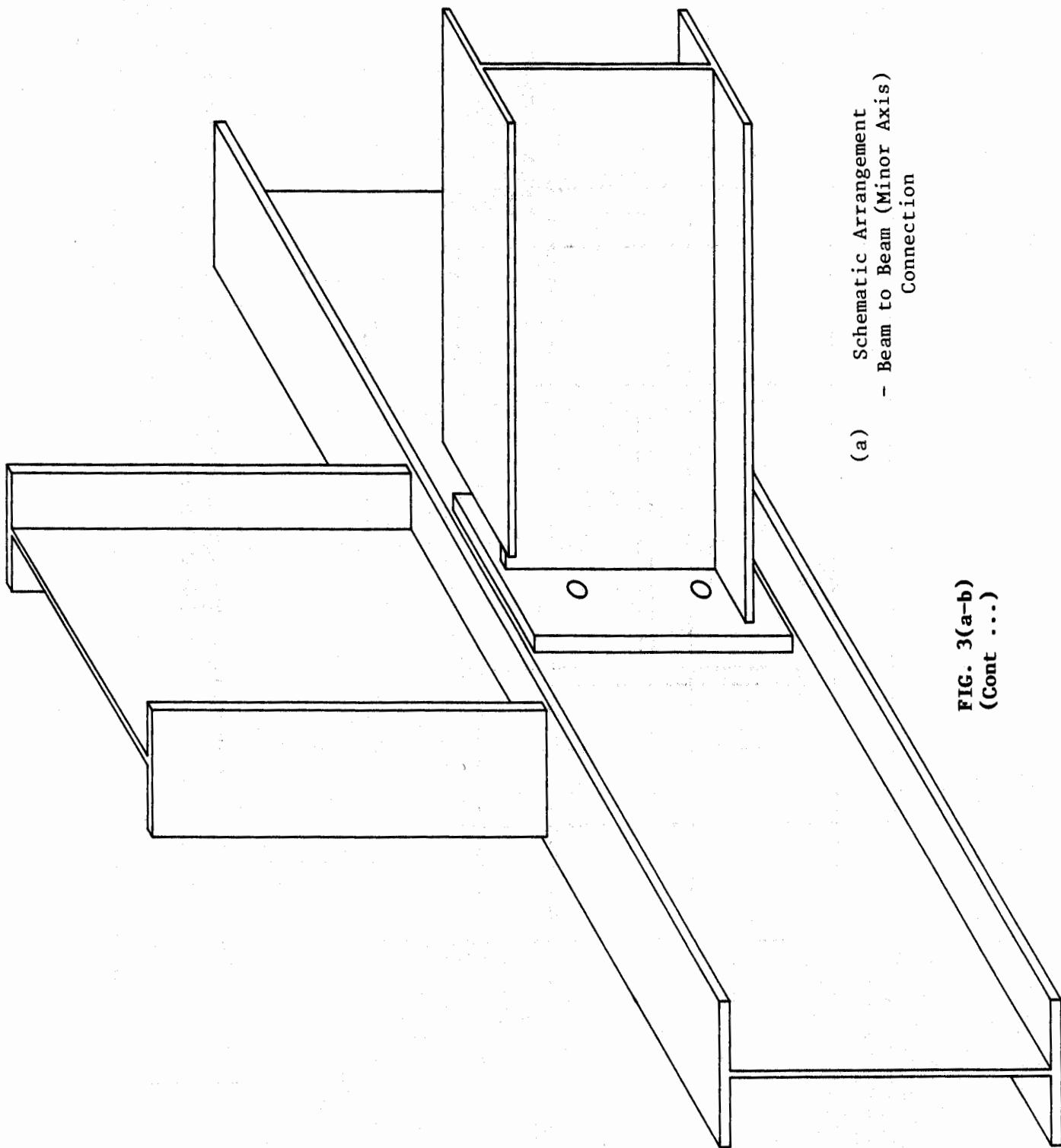
FIG. 1





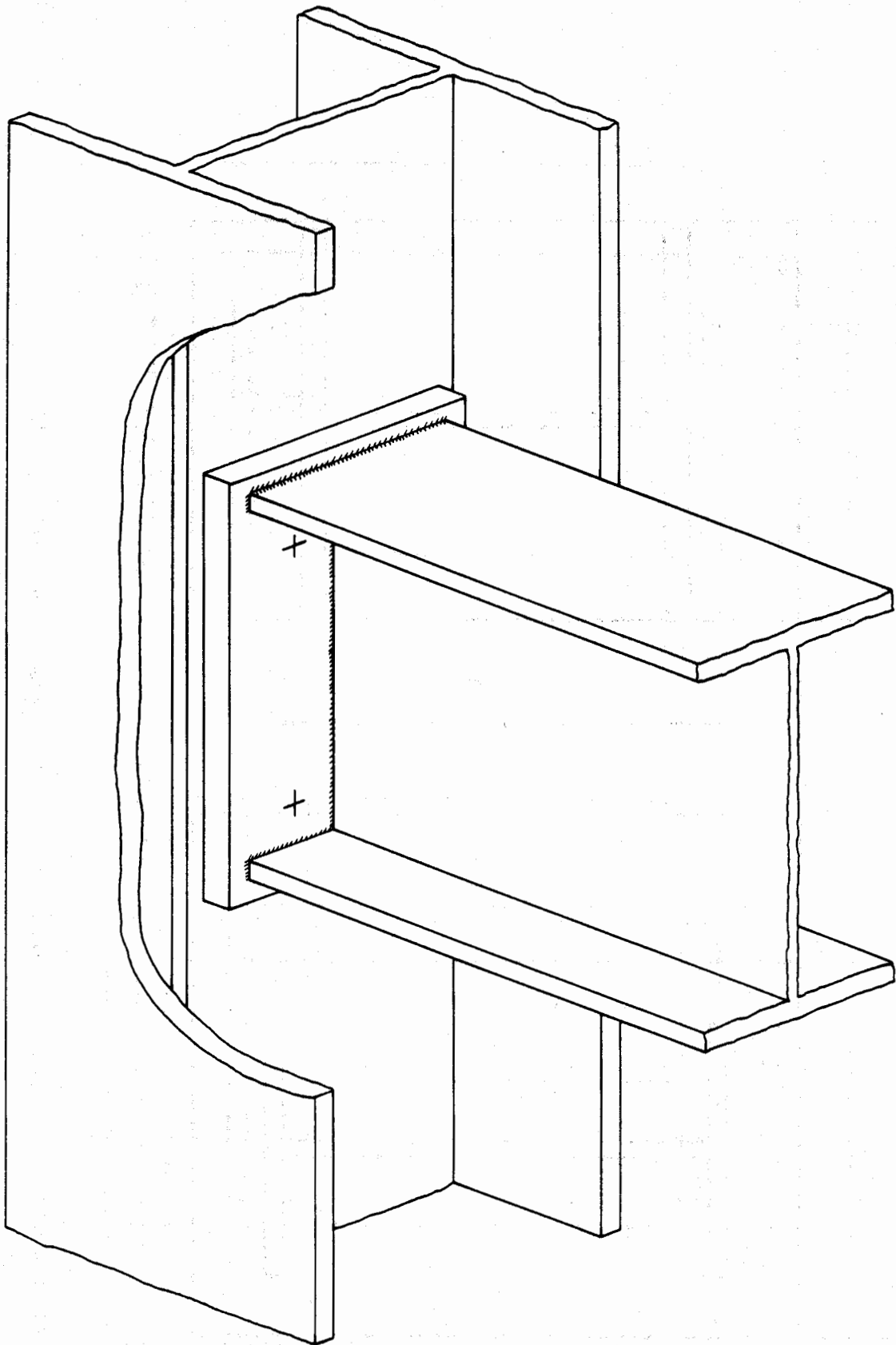
(R4/3641)

FIG. 2 DIMENSIONAL DATA FOR COLUMN TOP PLATES (ASSEMBLIES 1-8)



(a) Schematic Arrangement  
- Beam to Beam (Minor Axis)  
Connection

FIG. 3(a-b)  
(Cont ...)



(b) Schematic Arrangement - Beam to Column (R4/3642)  
(Minor Axis) Connection  
(Column Flange Cut Away for Clarity)

FIG. 3(a-b)

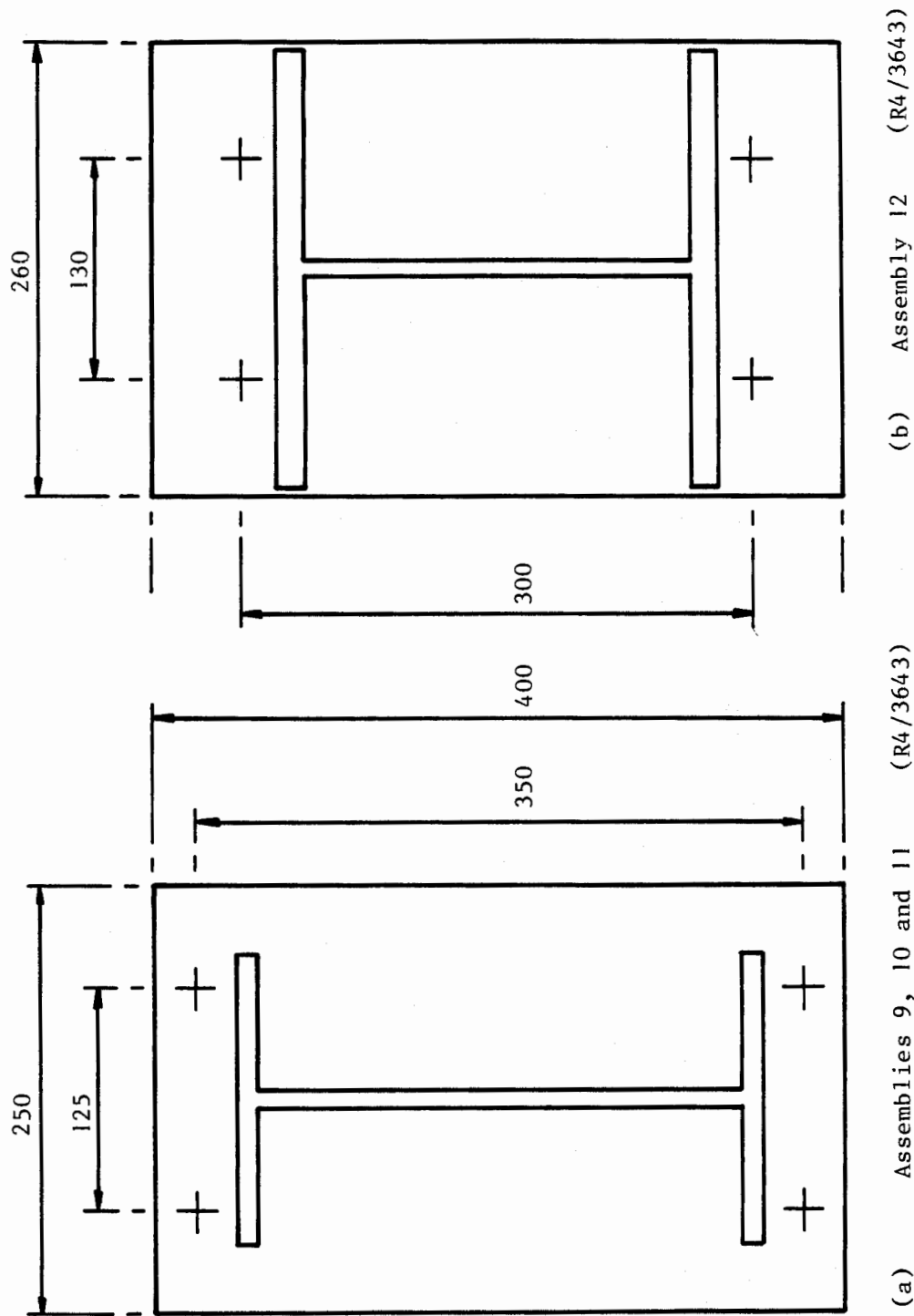


FIG. 4(a-b) DIMENSIONAL DATA FOR COLUMN TOP PLATES (ASSEMBLIES 9-12)

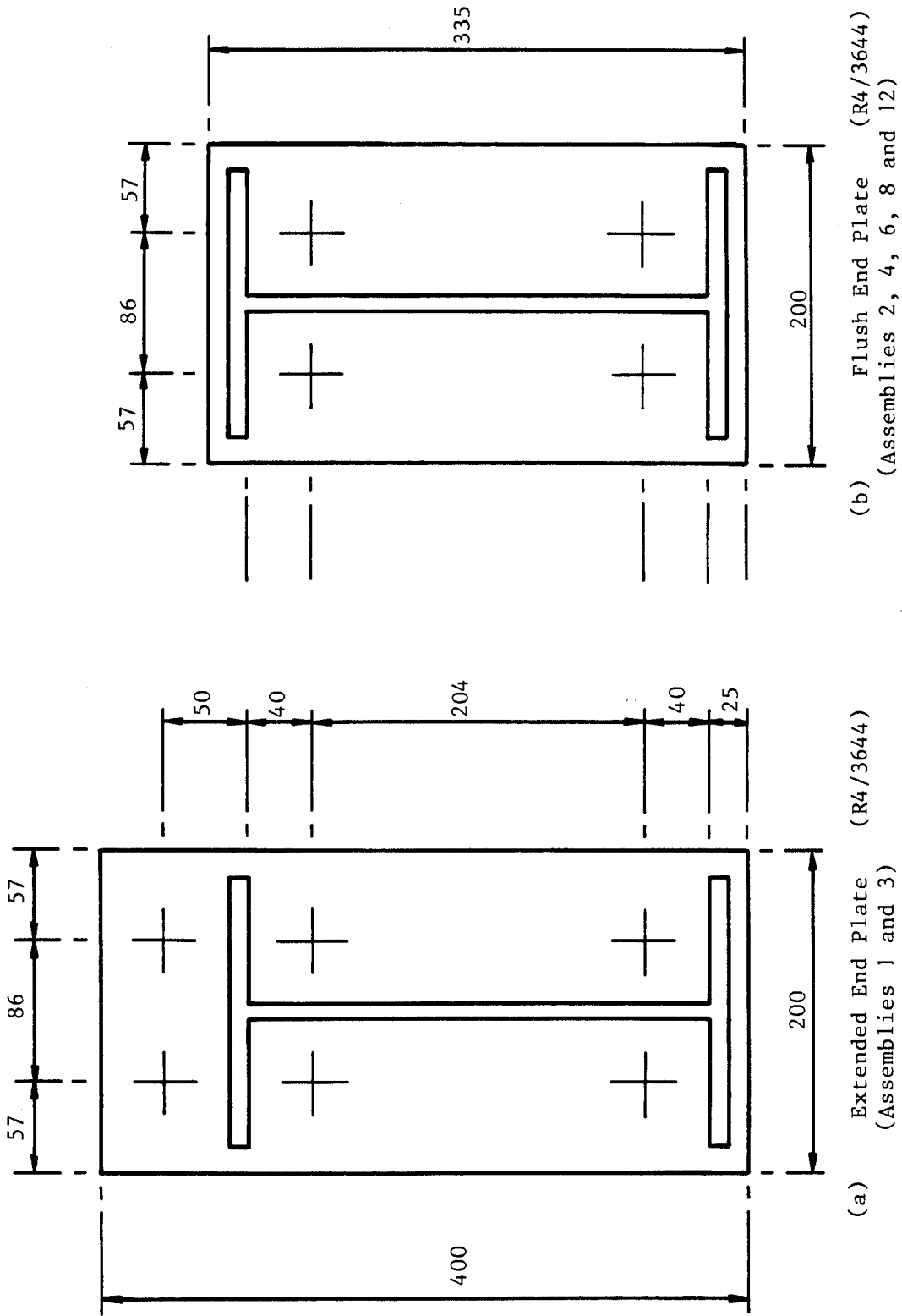


FIG. 5(a-b) DIMENSIONAL DATA FOR END PLATES

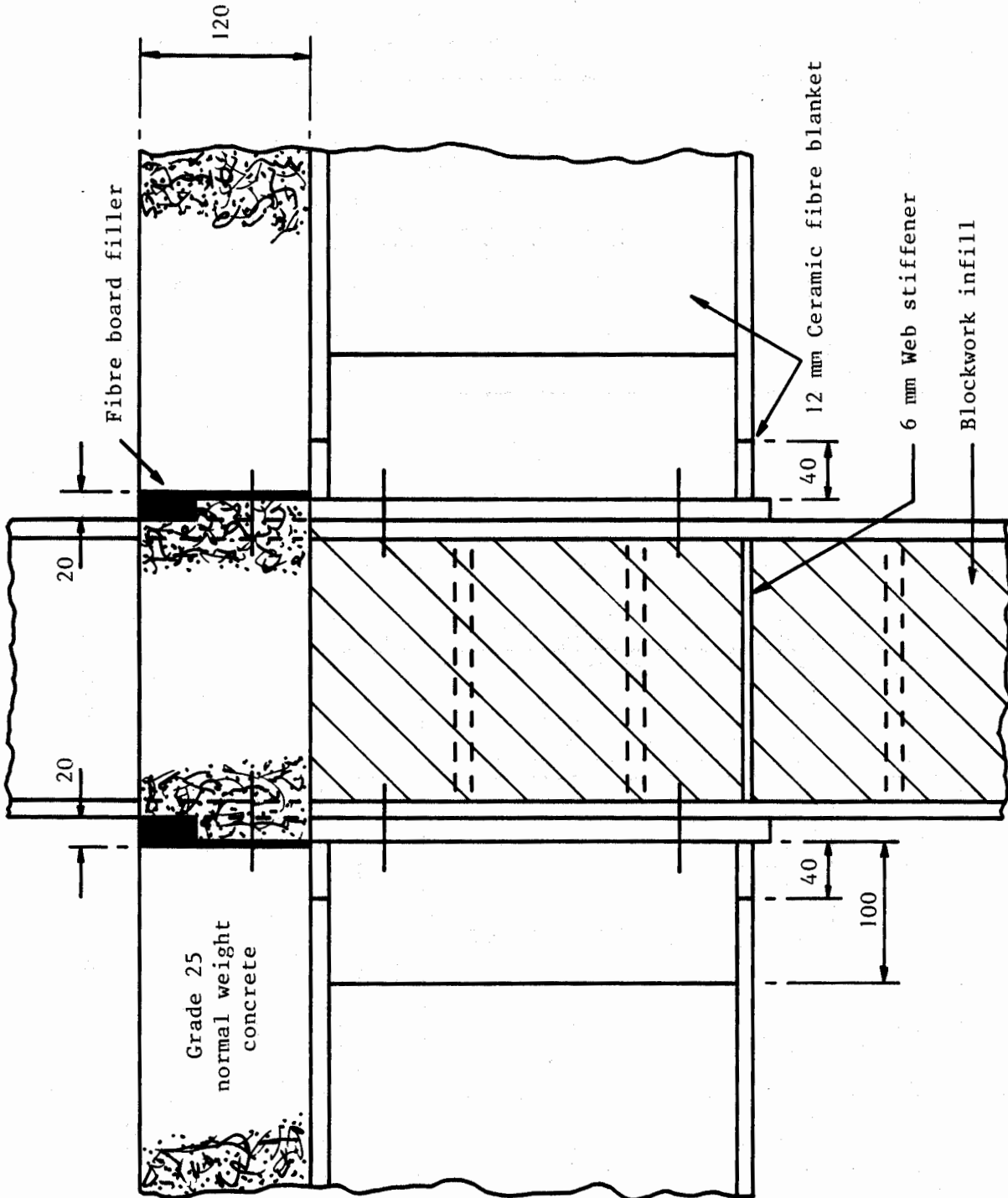


FIG. 6 CONSTRUCTION DETAILS FOR ASSEMBLY NO. 1 (R4/3645)

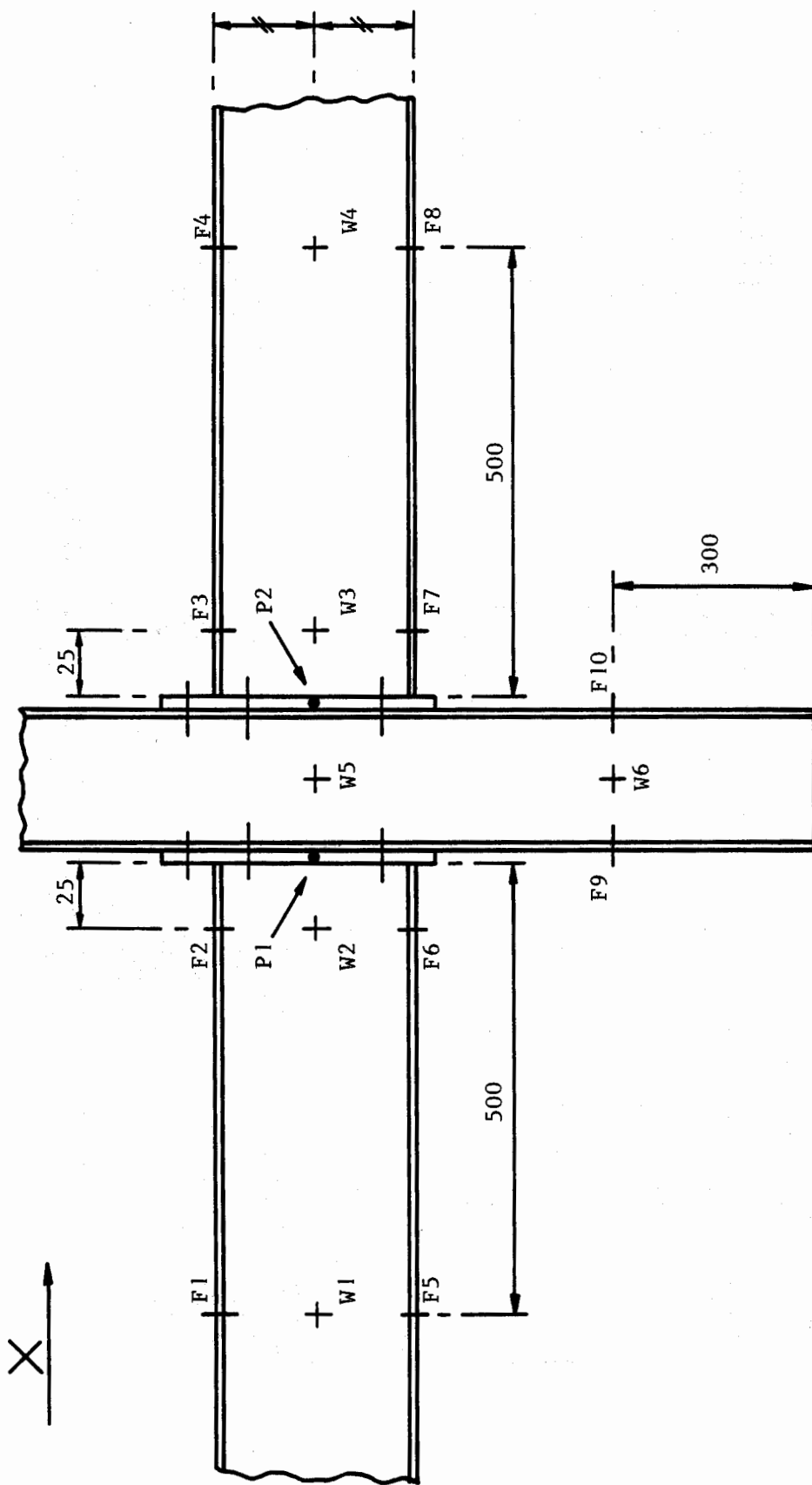
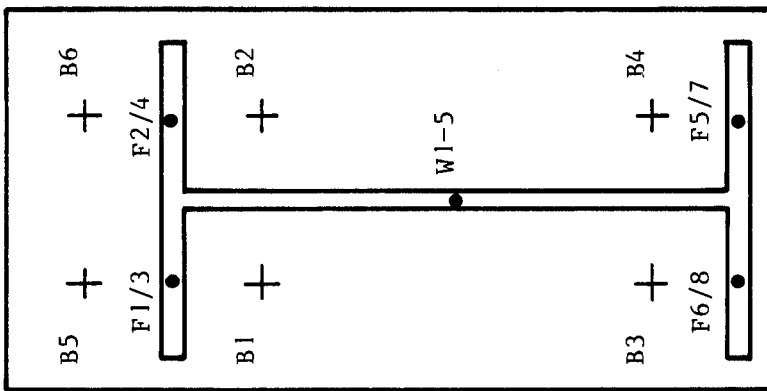
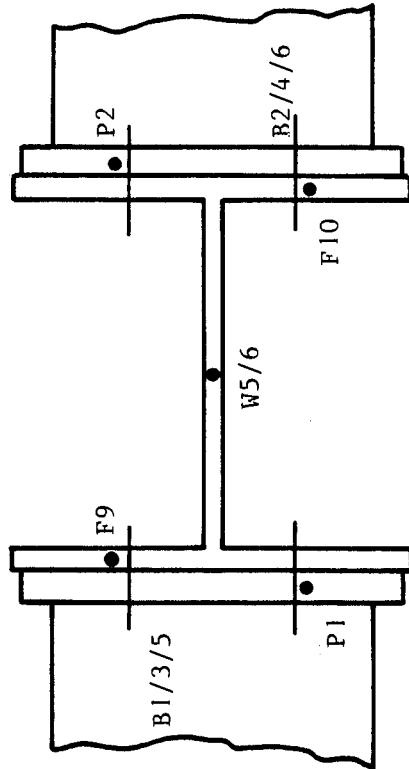


FIG. 7 THERMOCOUPLE POSITIONS - ASSEMBLY NO. 1 (R4/3646)  
(LONGITUDINAL ARRANGEMENT)



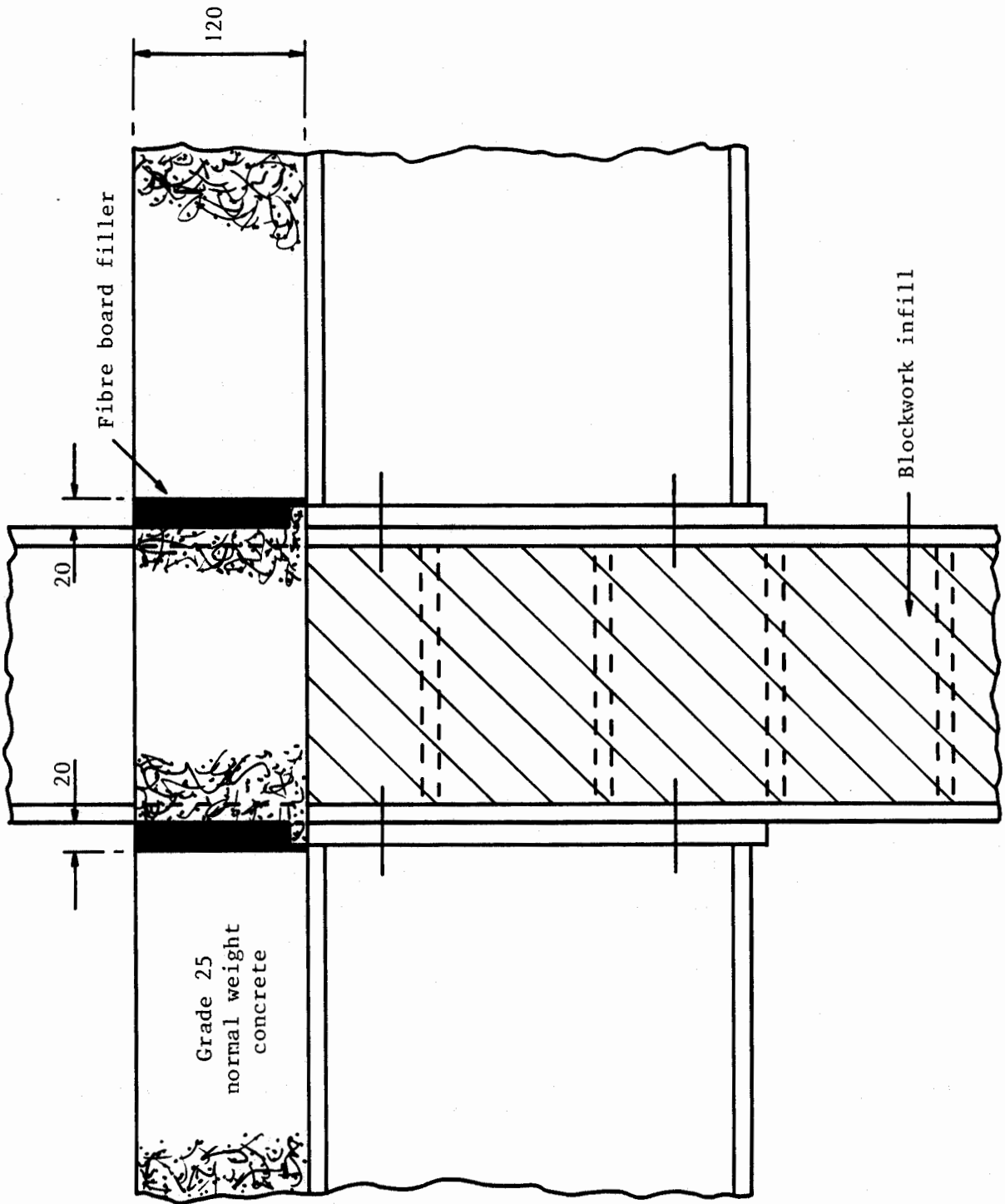
View in Direction of  
Arrow X in Fig. 7



Plan

FIG. 8 THERMOCOUPLE POSITIONS - ASSEMBLY NO. 1  
(TRANSVERSE AND PLAN ARRANGEMENTS) (R4/3647)





(R4/3648)

CONSTRUCTION DETAILS FOR ASSEMBLY NO. 2

FIG. 9

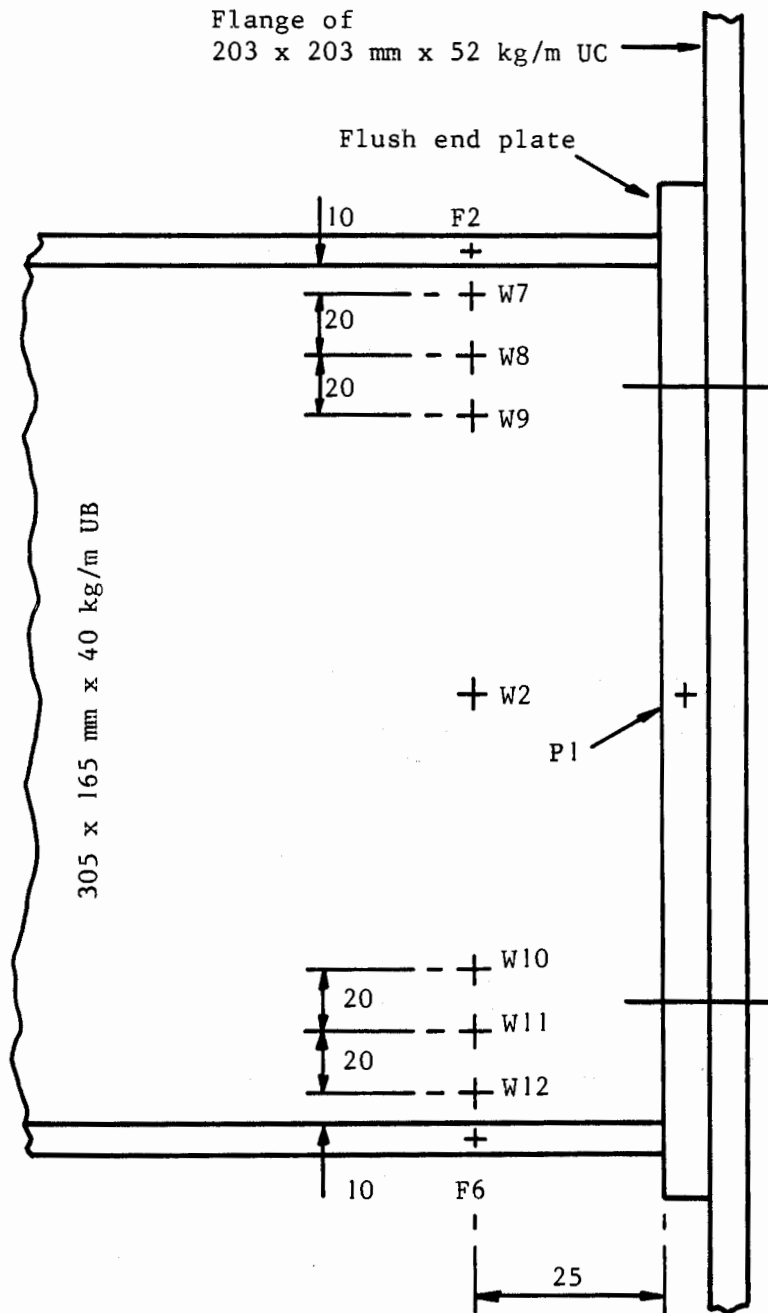


FIG. 10

ADDITIONAL THERMOCOUPLE POSITIONS IN THE WEB  
- ASSEMBLY NO. 2

(R4/3649)

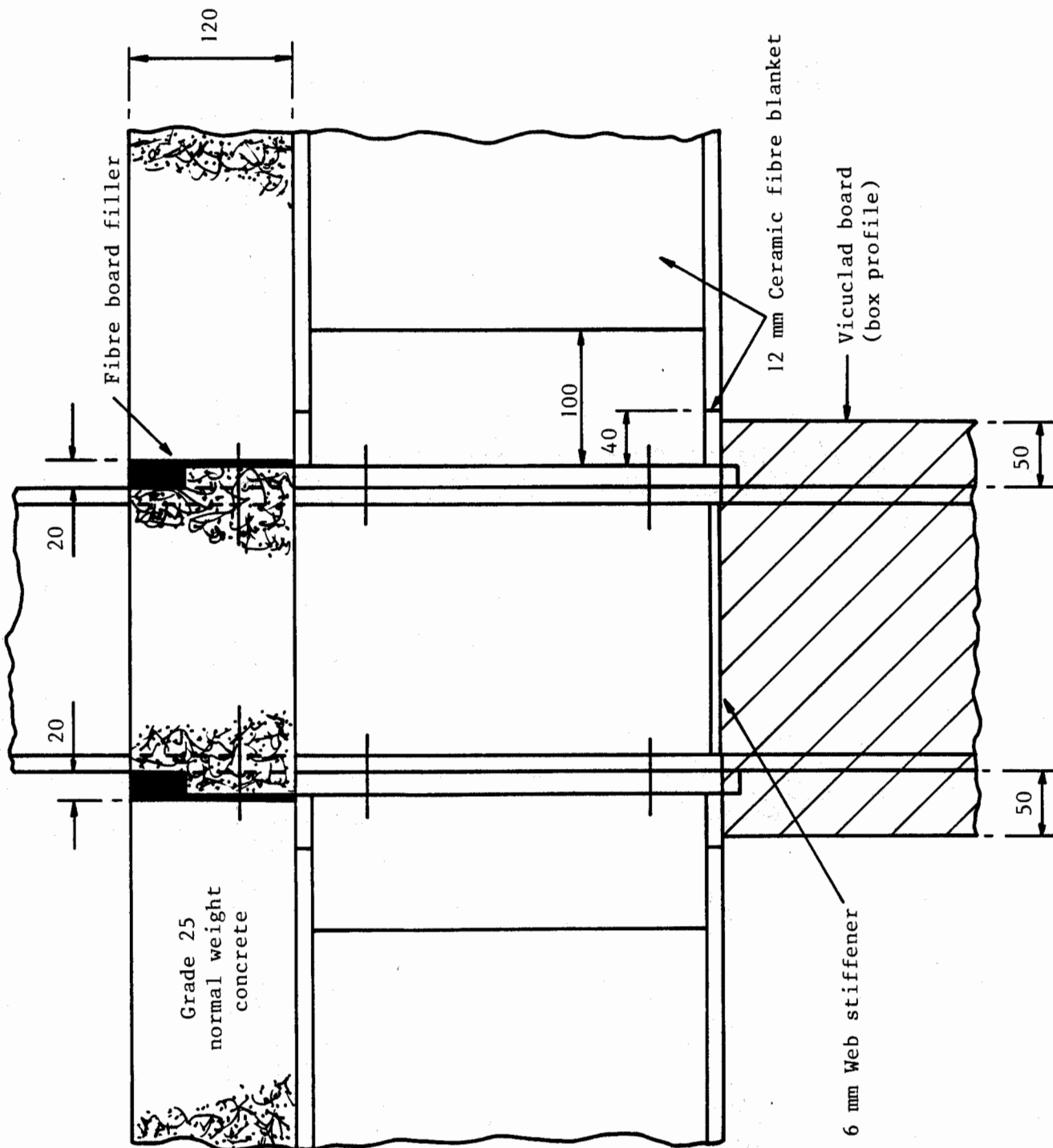


FIG. 11 CONSTRUCTION DETAILS FOR ASSEMBLY NO. 3 (R4/3650)

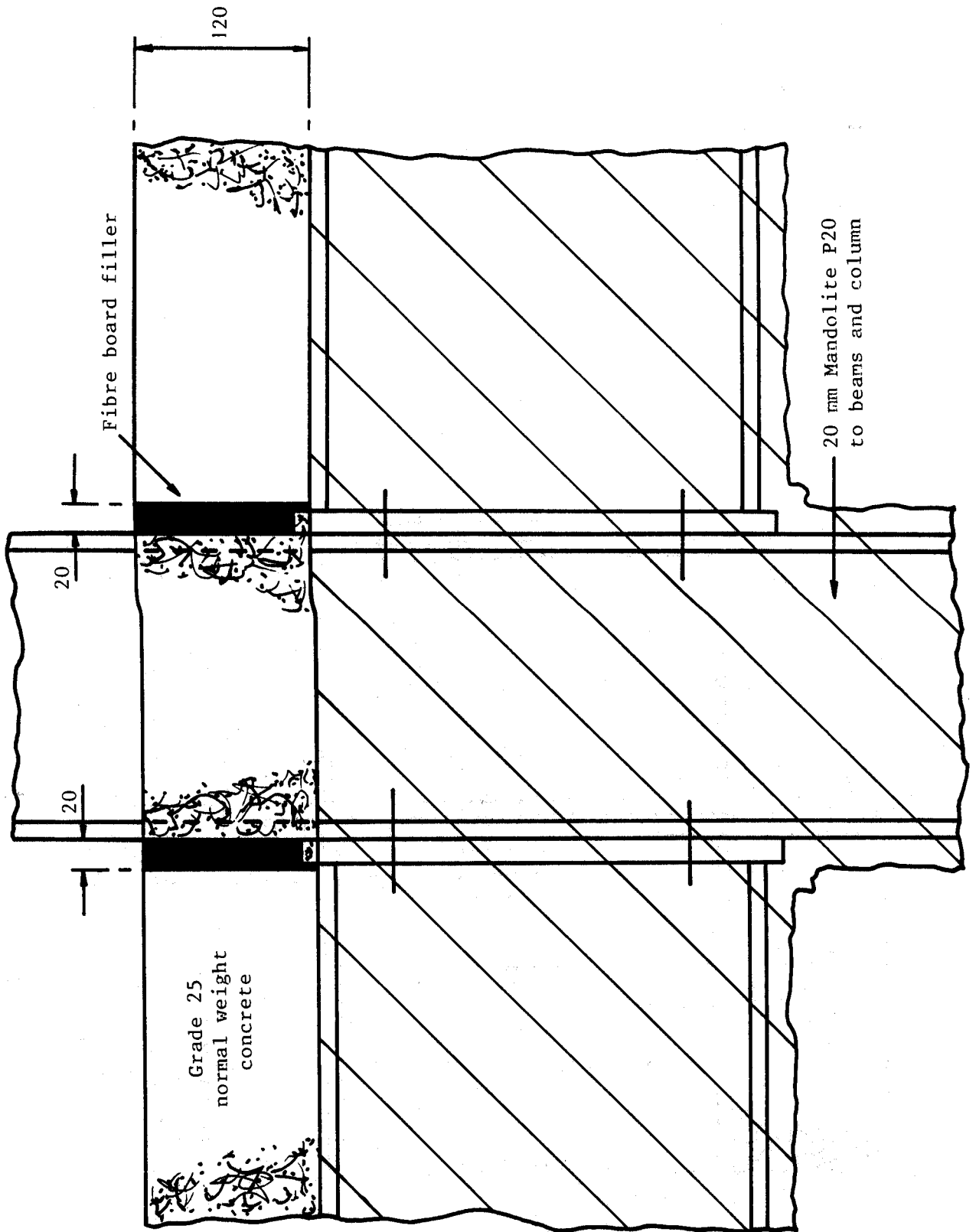


FIG. 12 CONSTRUCTION DETAILS FOR ASSEMBLY NO. 4 (R4/3651)

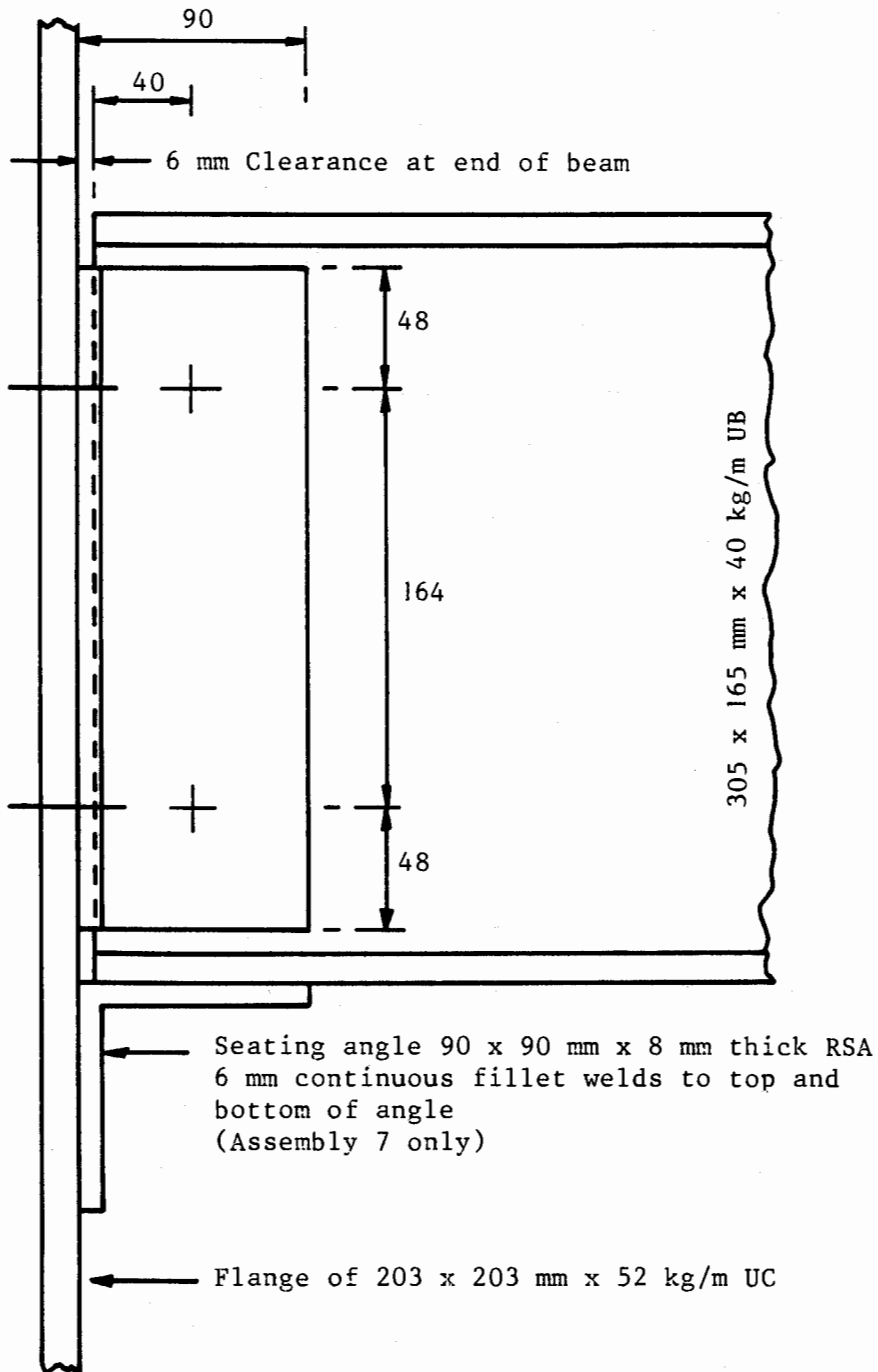


FIG. 13

CONSTRUCTION DETAILS FOR ASSEMBLY NO. 5

(R4/3652)

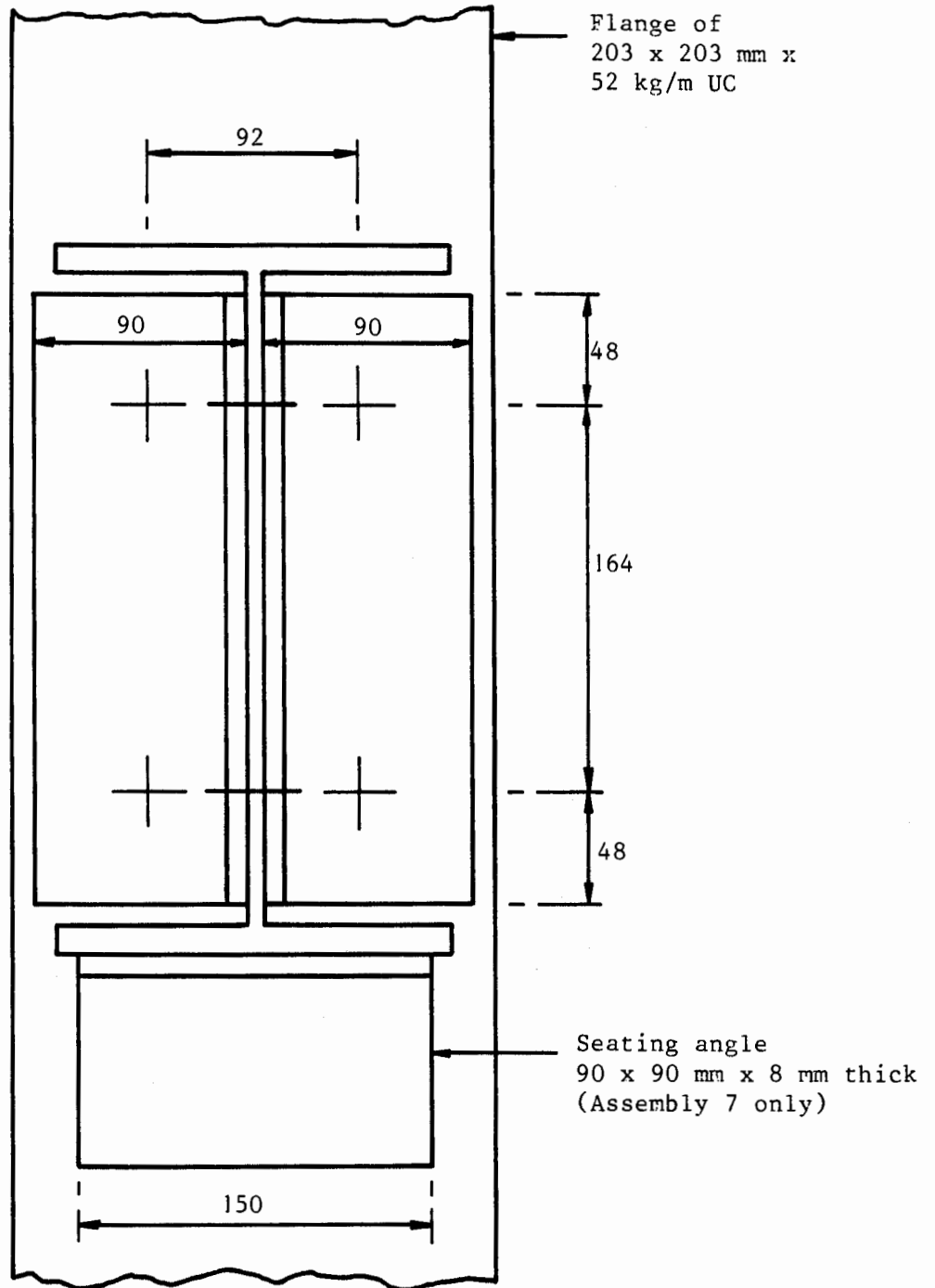


FIG. 14

CONSTRUCTION DETAILS FOR ASSEMBLY NO. 5

(R4/3653)

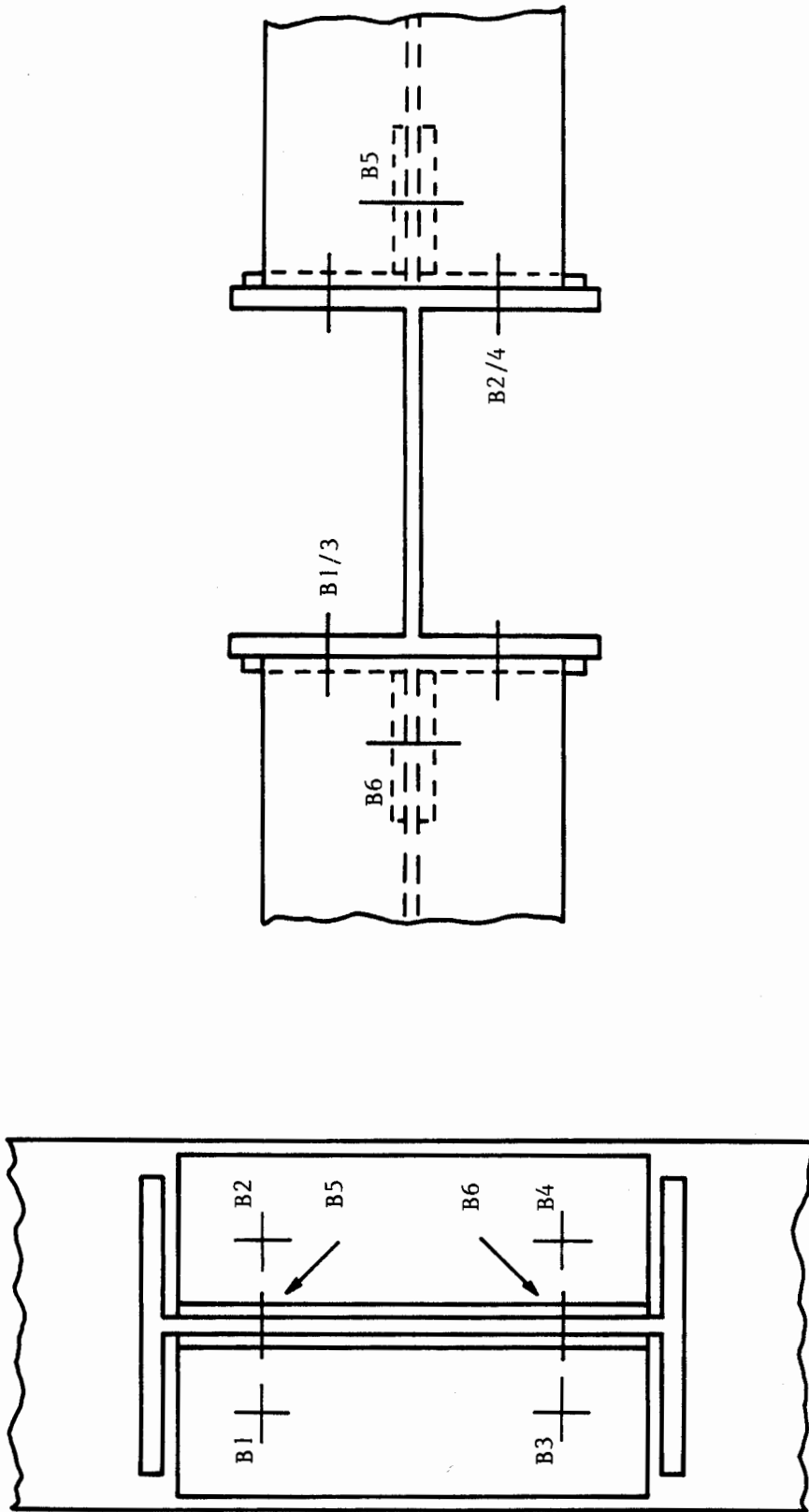
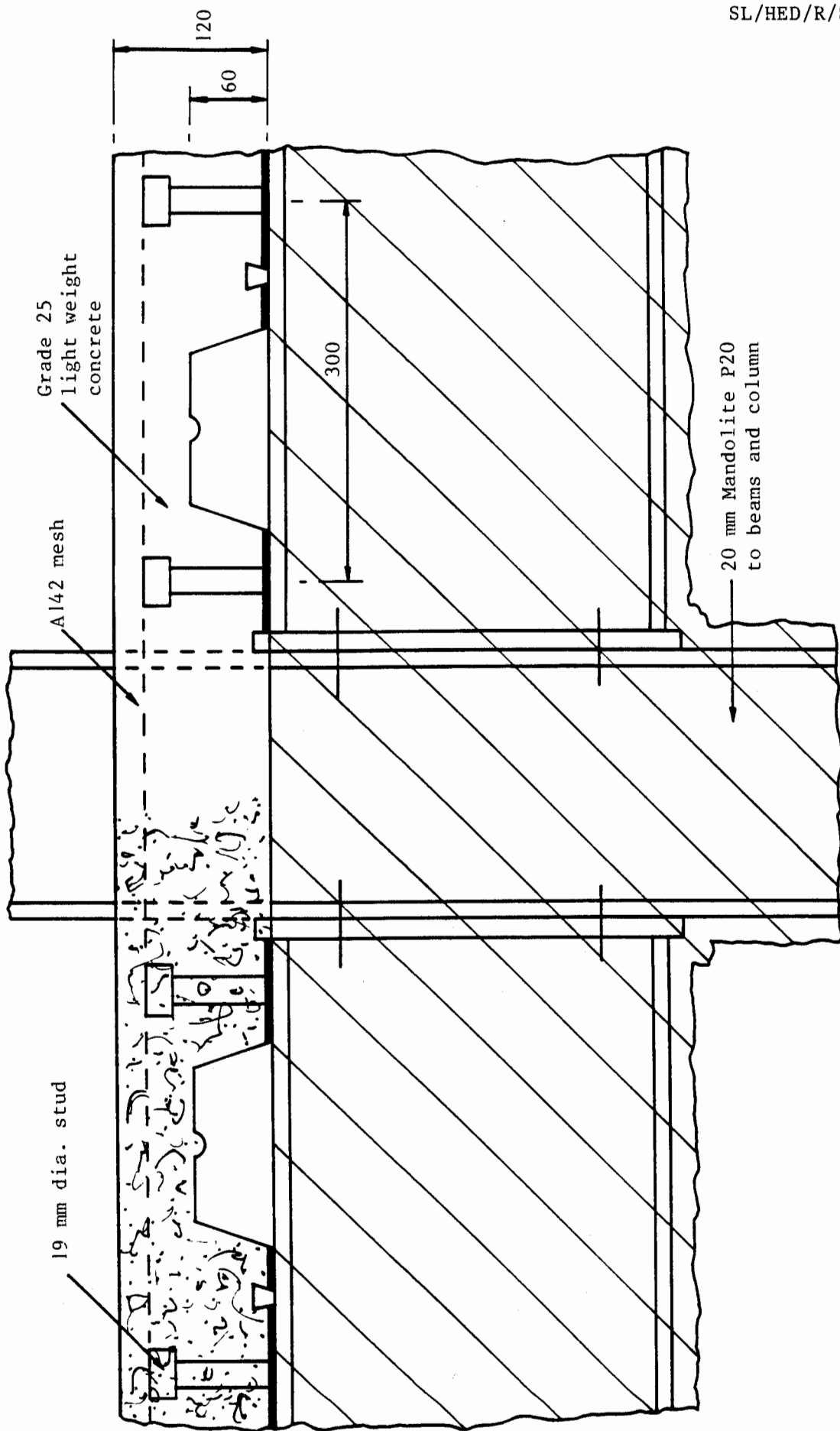


FIG. 15 THERMOCOUPLE POSITIONS IN THE BOLTS - ASSEMBLY NO. 5 (R4/3654)

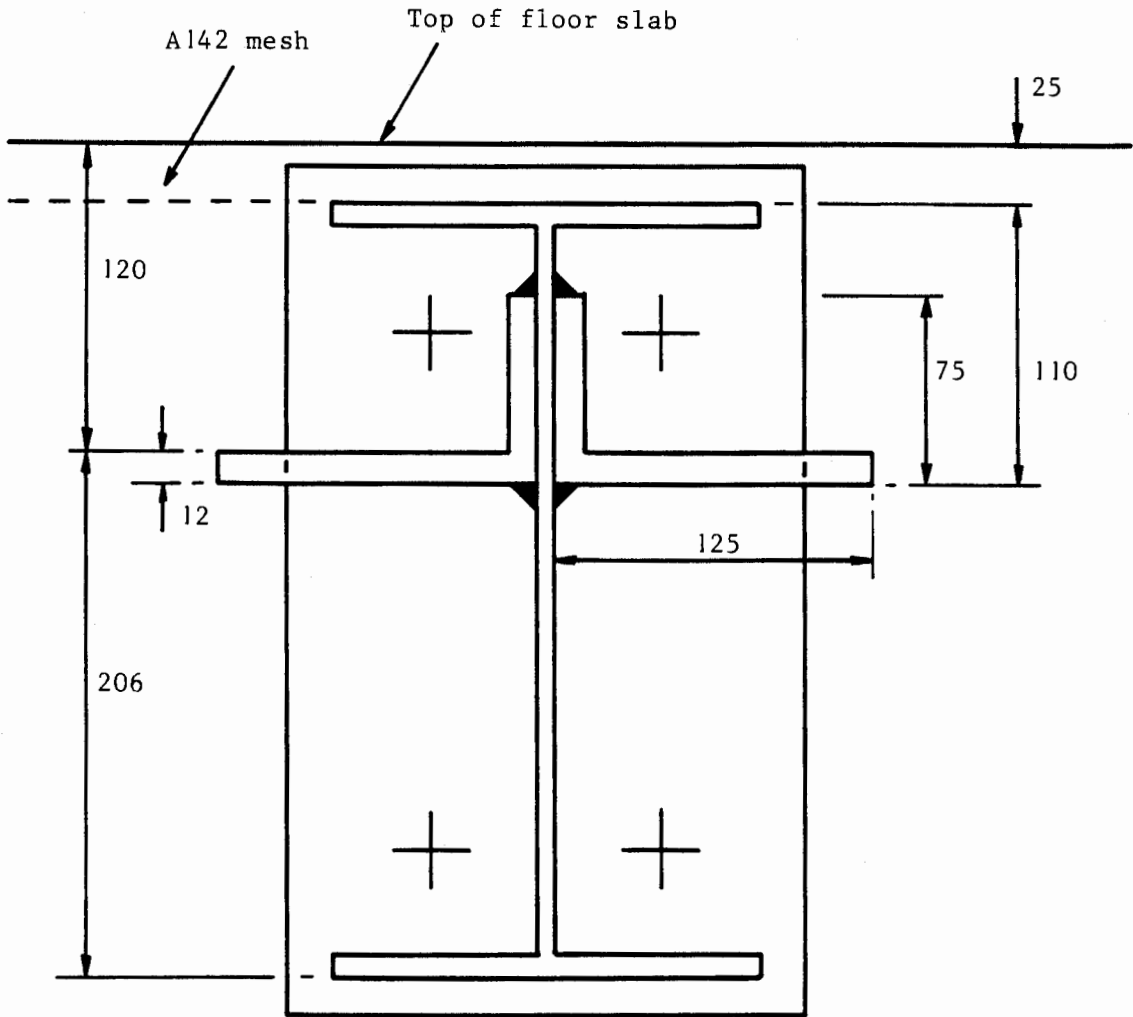


(R4/3655)

CONSTRUCTION DETAILS FOR ASSEMBLY NO. 6

FIG. 16



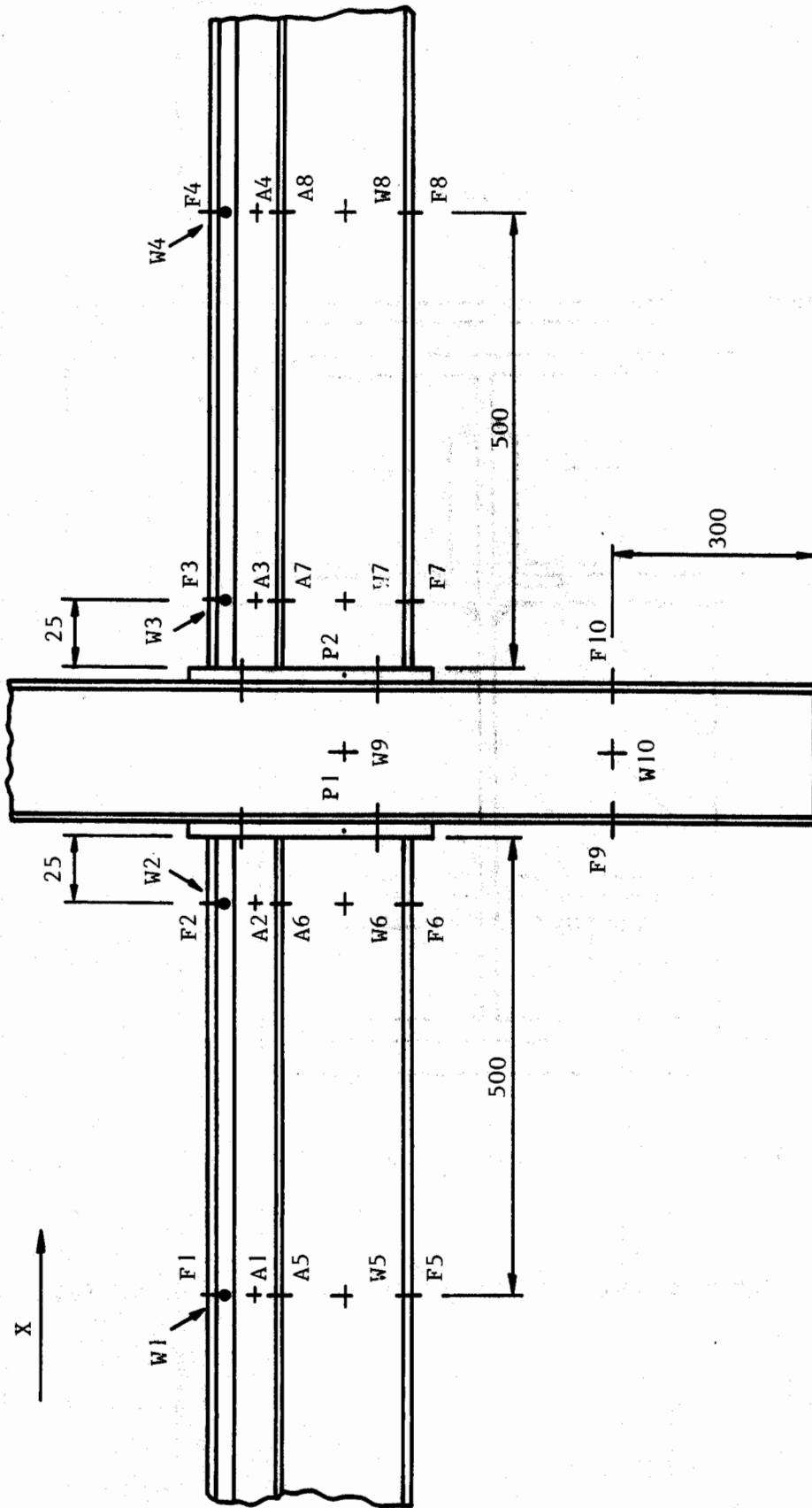


Flush end plate details shown in Fig. 5(b)

FIG. 17

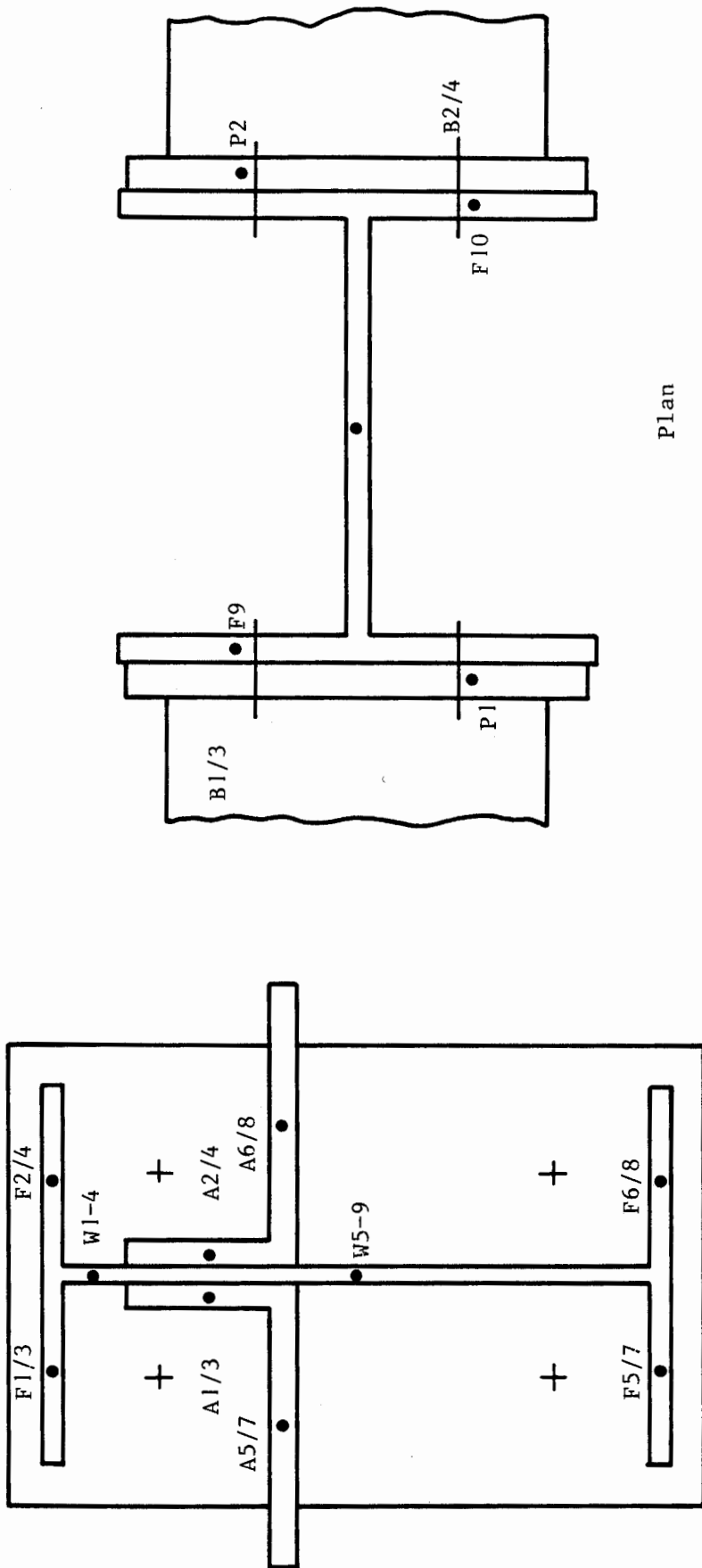
CONSTRUCTION DETAILS FOR ASSEMBLY NO. 8

(R4/3656)



(R4/3657)

FIG. 18 THERMOCOUPLE POSITIONS - ASSEMBLY NO. 8  
(LONGITUDINAL ARRANGEMENT)



View in Direction of  
Arrow X in Fig. 18

FIG. 19 THERMOCOUPLE POSITIONS - ASSEMBLY NO. 8  
(TRANSVERSE AND PLAN ARRANGEMENTS) (R4/3658)

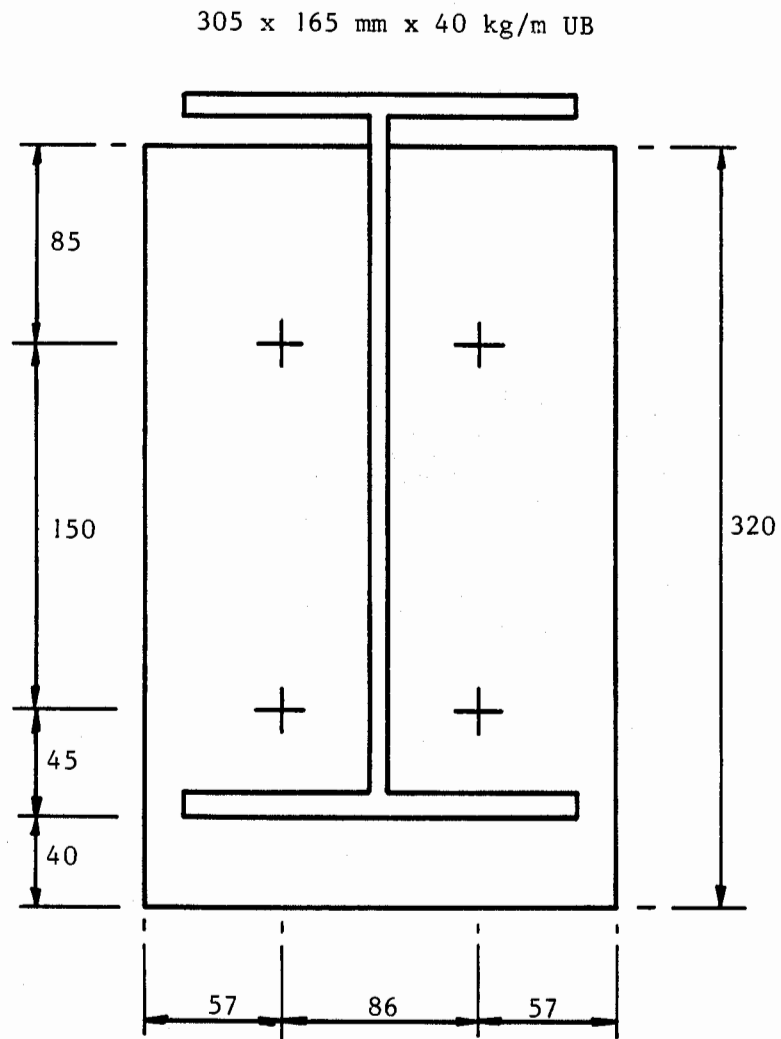


FIG. 20

DIMENSIONAL DATA FOR END PLATES  
- ASSEMBLY NO. 9

(R4/3659)

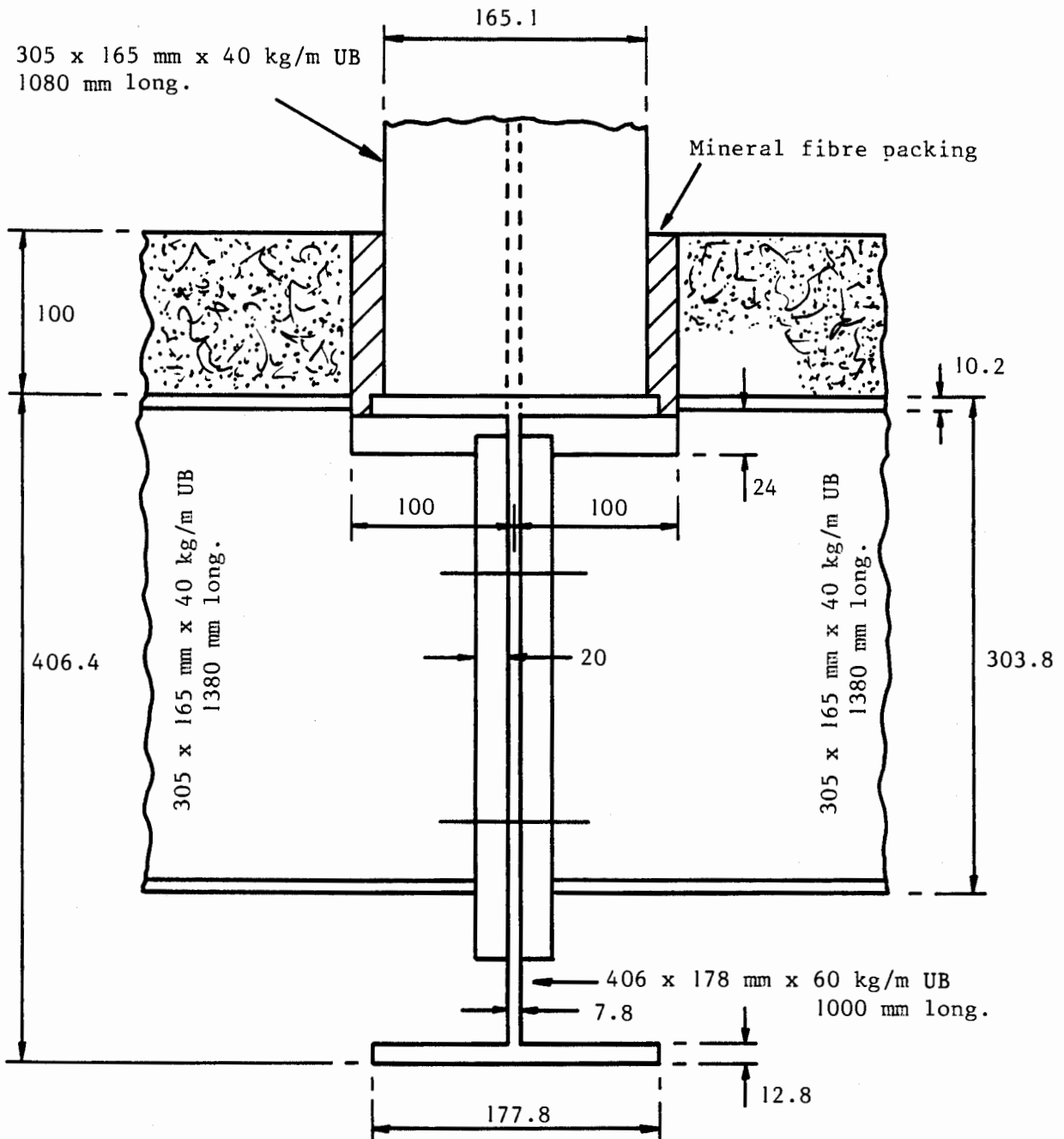


FIG. 21

CONSTRUCTION DETAILS FOR ASSEMBLY NO. 9

(R4/3660)

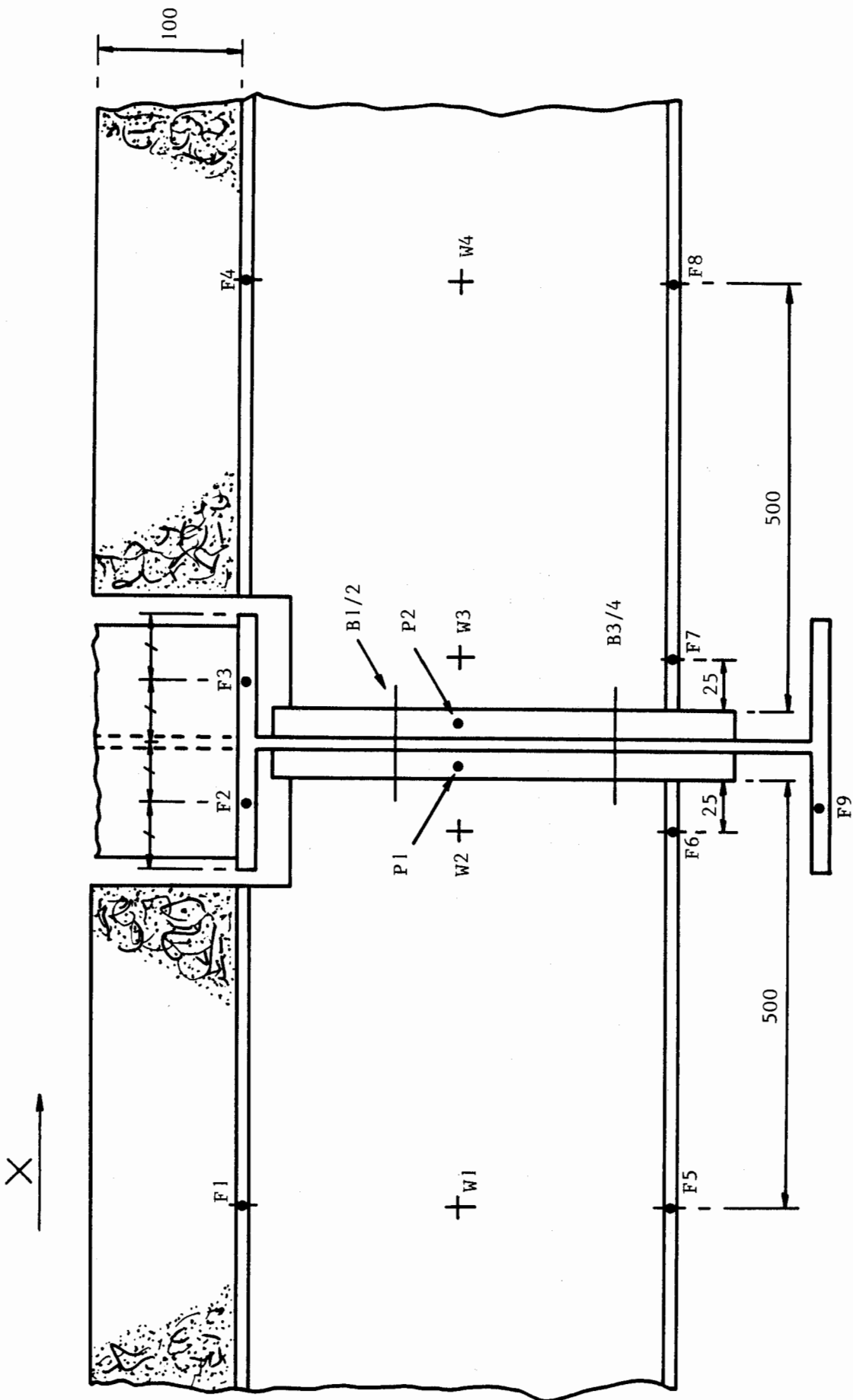
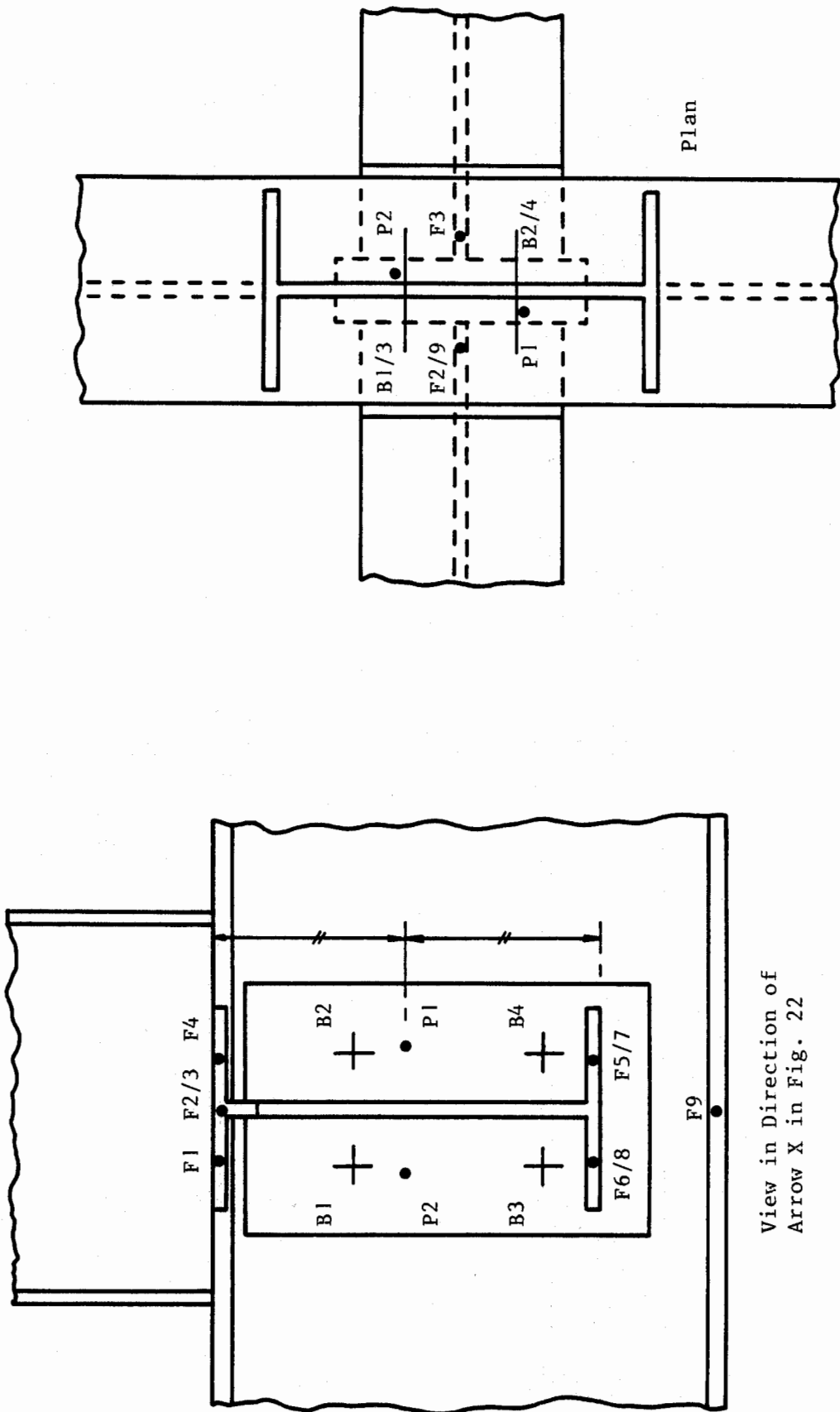


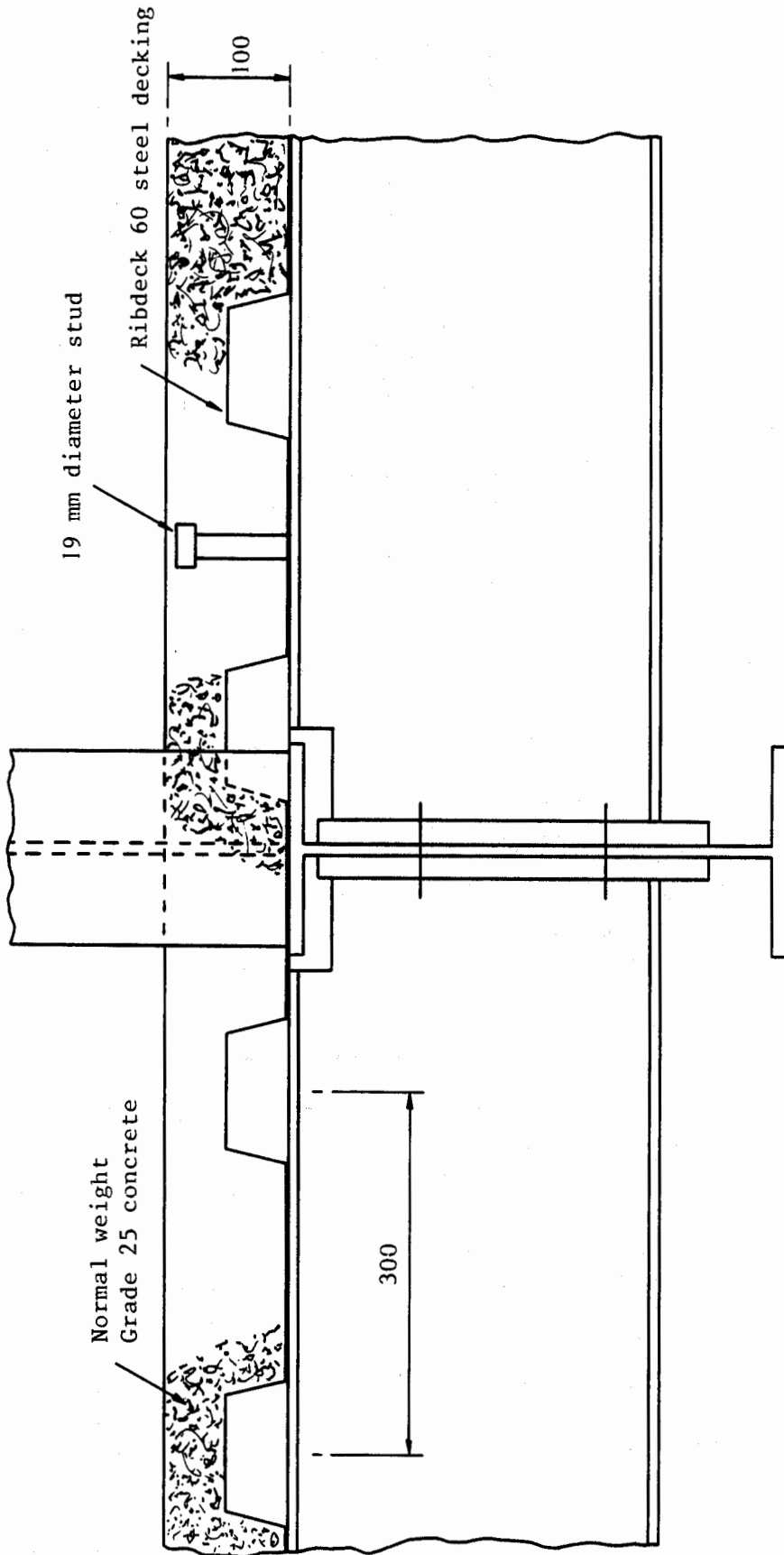
FIG. 22 THERMOCOUPLE POSITIONS - ASSEMBLY NO. 9 (R4/3661)  
(LONGITUDINAL ARRANGEMENT)



(R4/3662)

FIG. 23 THERMOCOUPLE POSITIONS - ASSEMBLY NO. 9  
(TRANSVERSE AND PLAN ARRANGEMENTS)

View in Direction of  
Arrow X in Fig. 22



All other dimensions as shown in Fig. 21

FIG. 24

CONSTRUCTION DETAILS FOR ASSEMBLY NO. 10

(R4/3663)



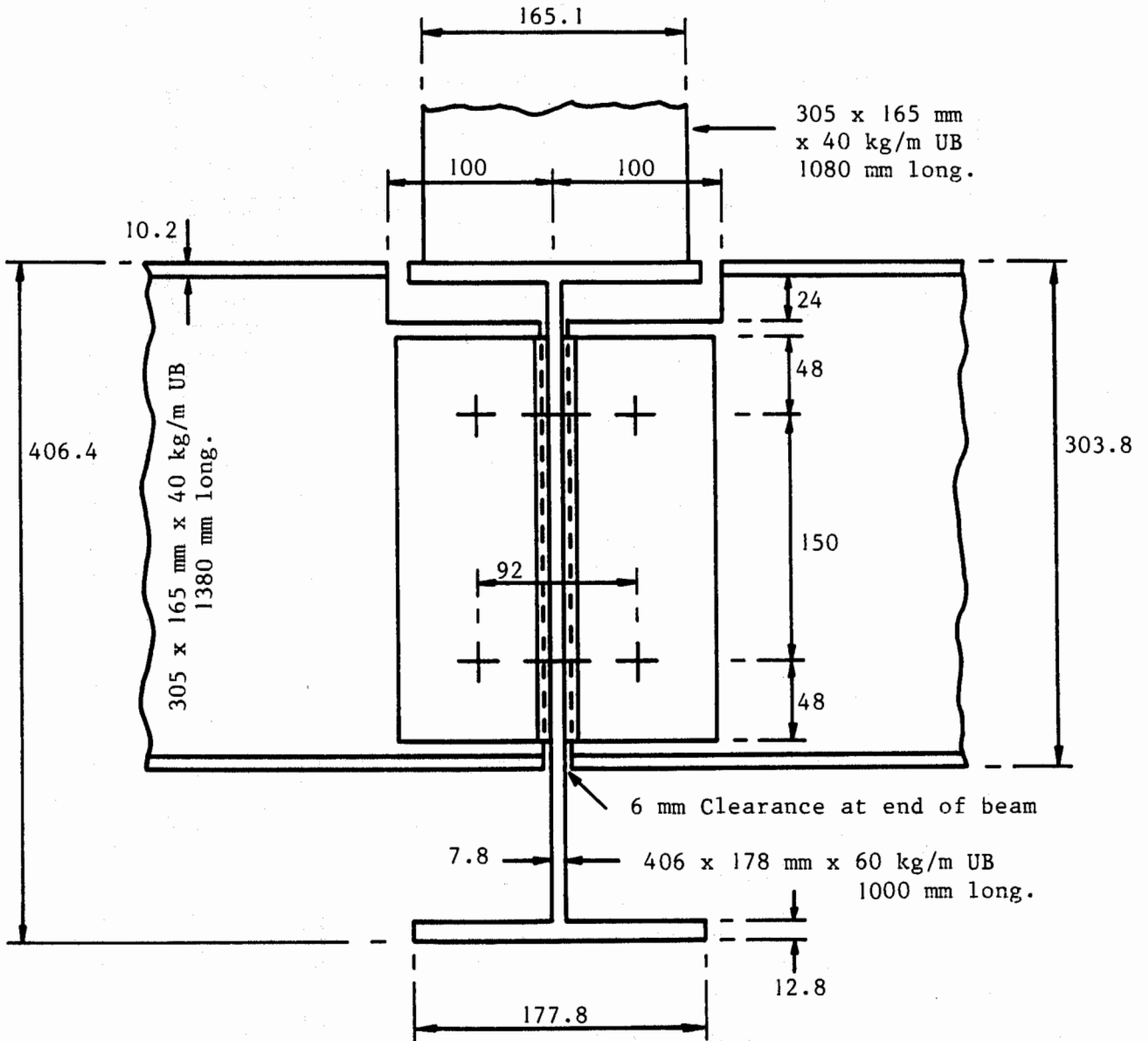
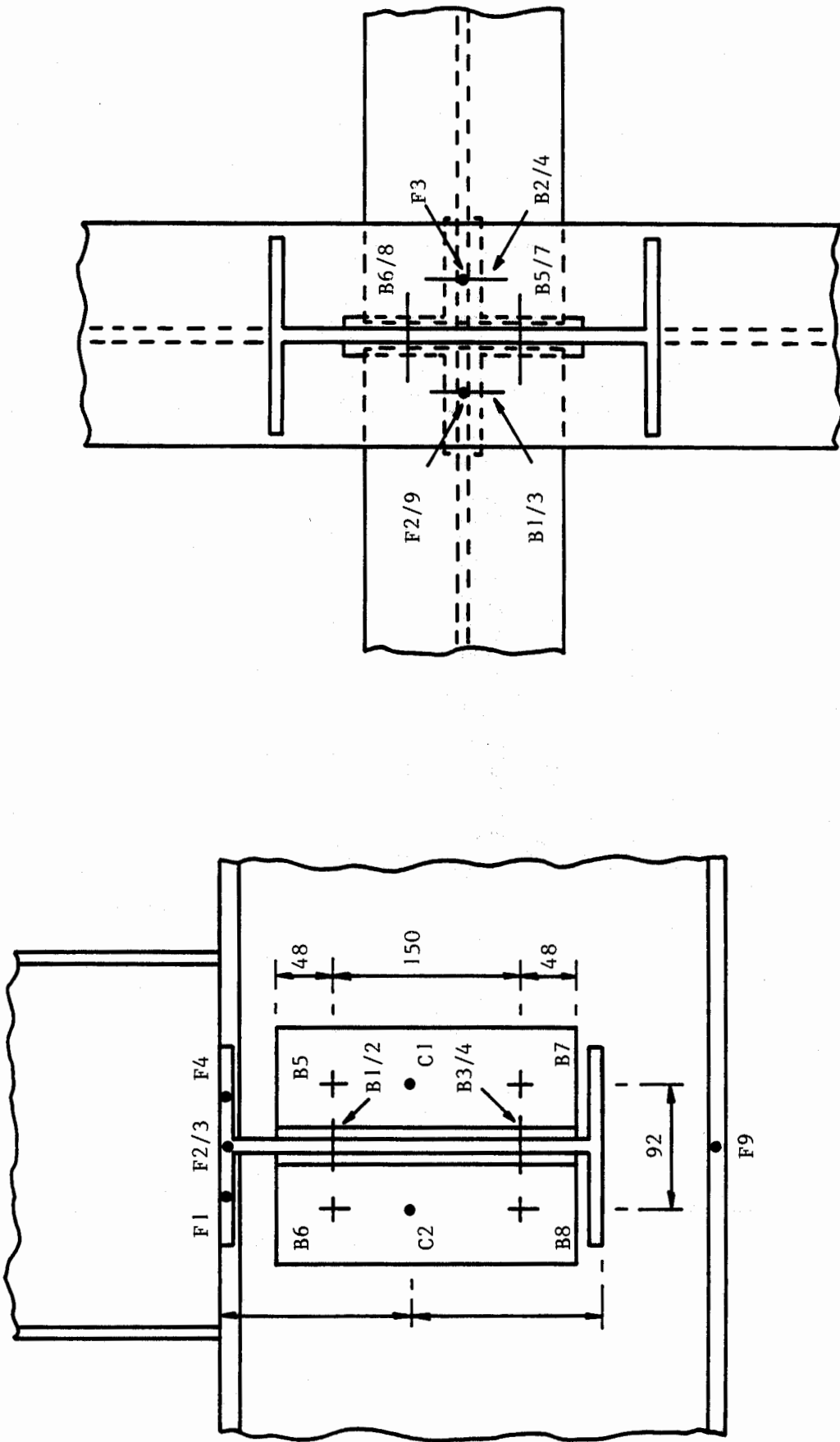


FIG. 25

CONSTRUCTION DETAILS FOR ASSEMBLY NO. 11

(R4/3664)



(R4/3665)

FIG. 26 THERMOCOUPLE POSITIONS IN THE BOLTS - ASSEMBLY NO. 11

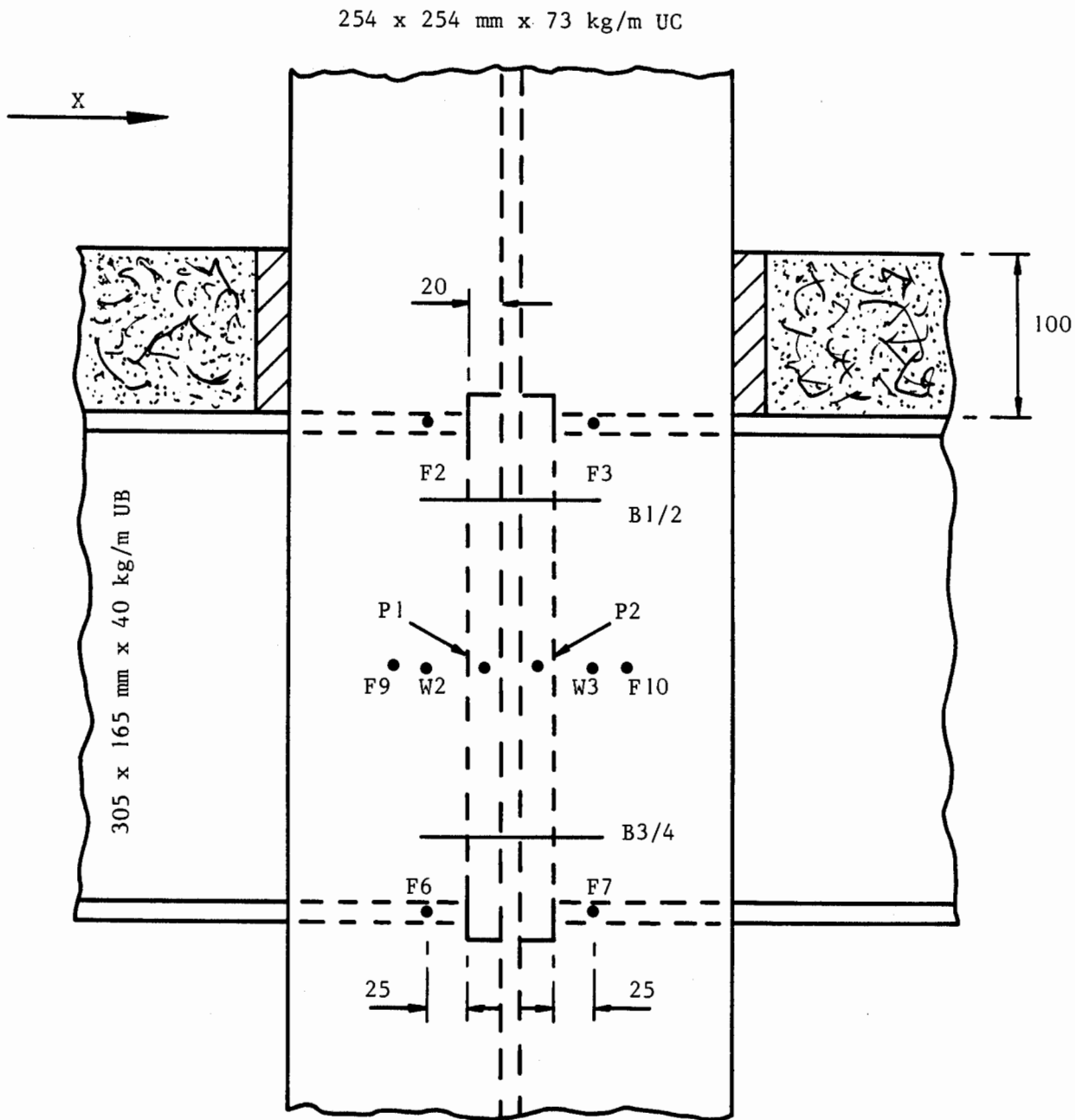
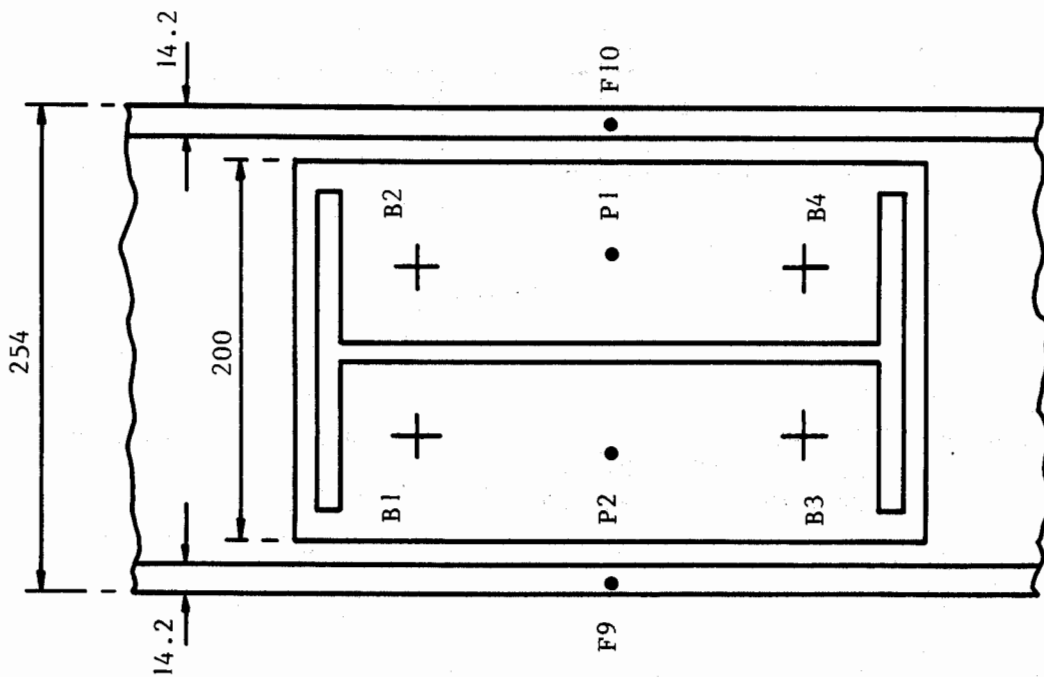


FIG. 27

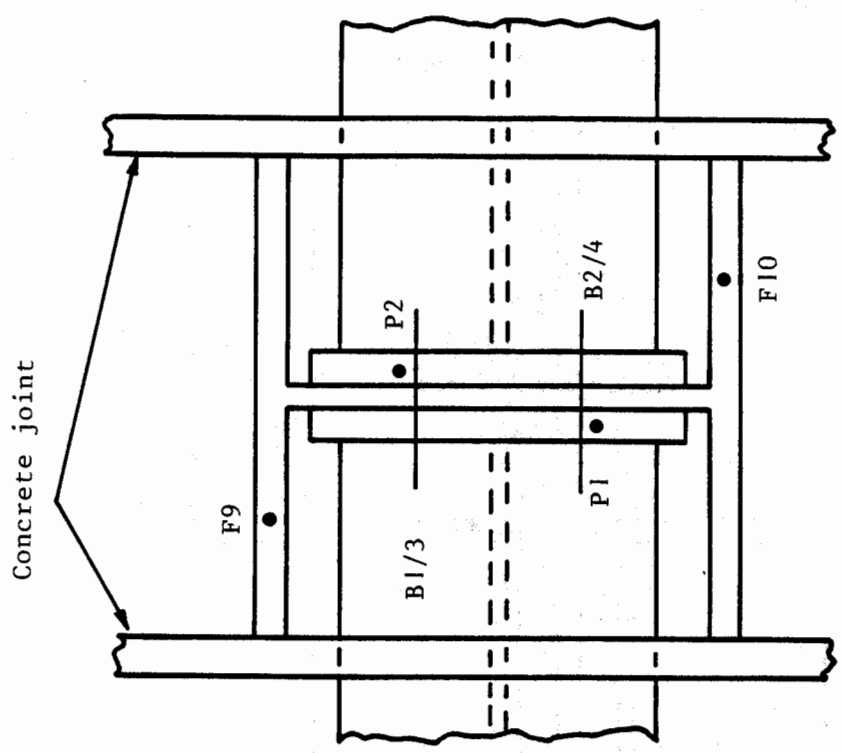
**THERMOCOUPLE POSITIONS AROUND THE CONNECTED  
MEMBERS - ASSEMBLY NO. 12  
(LONGITUDINAL ARRANGEMENT)**

(R4/3666)



(a) Side Elevation - View in Direction of Arrow X in Fig. 27

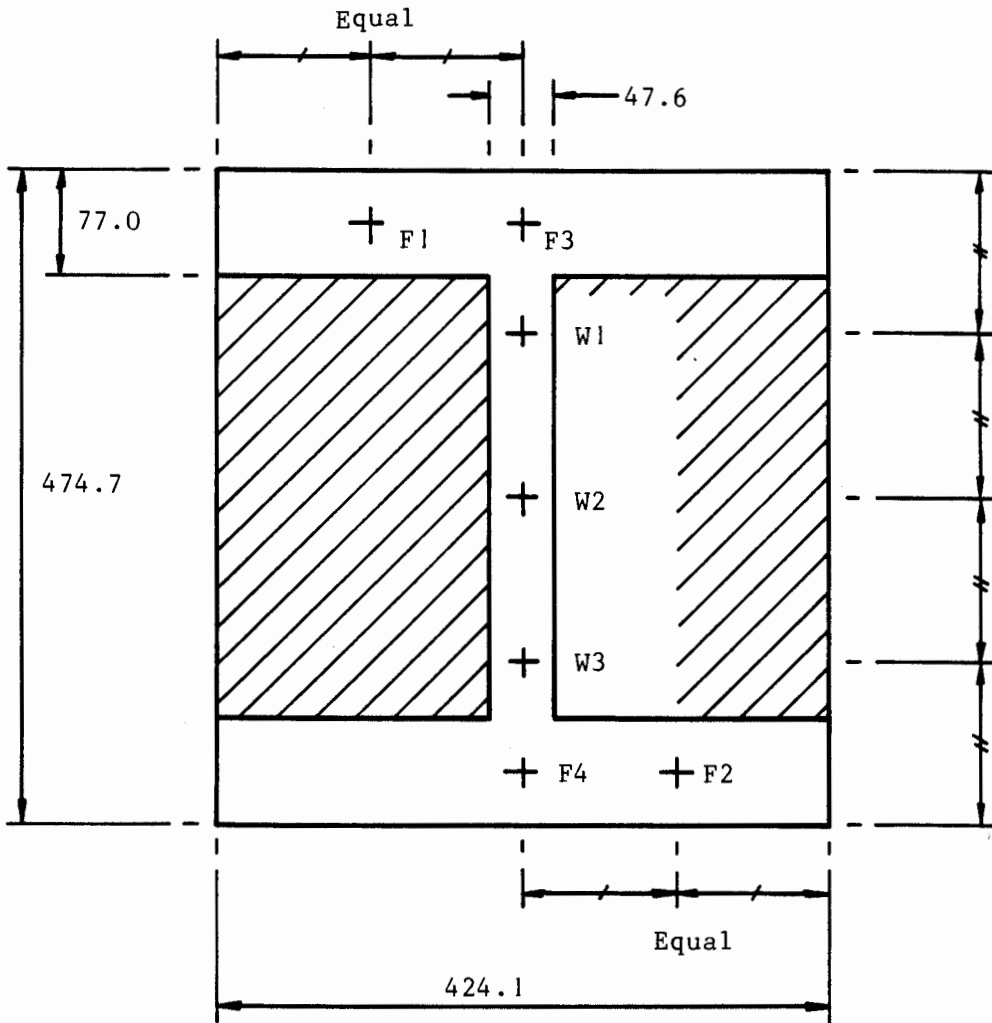
(R4/3667)



(b) Plan View

(R4/3667)

FIG. 28(a-b) THERMOCOUPLE POSITIONS - ASSEMBLY NO. 12 (TRANSVERSE AND PLAN ARRANGEMENTS)



Plan View

FIG. 29

THERMOCOUPLE POSITIONS IN THE INDICATIVE  
COLUMN SPECIMEN

(R4/3668)



**APPENDIX 1**

**DATA SHEET NUMBERS 112-124**

DATA  
SHEET  
NUMBER

112A

BEAM / COLUMN (MAJOR AXIS) CONNECTION

DIMENSIONS AND PROPERTIES

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
203 × 203 Column	Nominal Actual	52	206.2	203.9	8.0	12.5	510.4	173.6	568.1	263.7	5263	1770
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column Beam	Grade 43A	0.23	0.04	0.90	0.024	0.019	0.04	<0.005	0.03	<0.005	0.08	<0.005	<0.005	0.0048
	Grade 43A	0.22	0.03	0.88	0.009	0.025	<0.02	<0.005	<0.02	<0.005	<0.02	<0.005	<0.005	0.0074

ROOM TEMPERATURE TENSILE PROPERTIES

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)
Column - Flange	288	501	31
Beam - Flange (Actual)	309	480	33
Beam - Flange (Mean)	295	467	34

TEST CONDITIONS

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NOTES

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TEST CENTRE : Warrington Research  
 TEST DATE : 18th January 1989  
 TEST NUMBER : WFRC 44175

## ASSESSMENT

LOAD BEARING CAPACITY  
 45 minutes at 0.4  $M_p$   
 49 minutes at 0.3  $M_p$

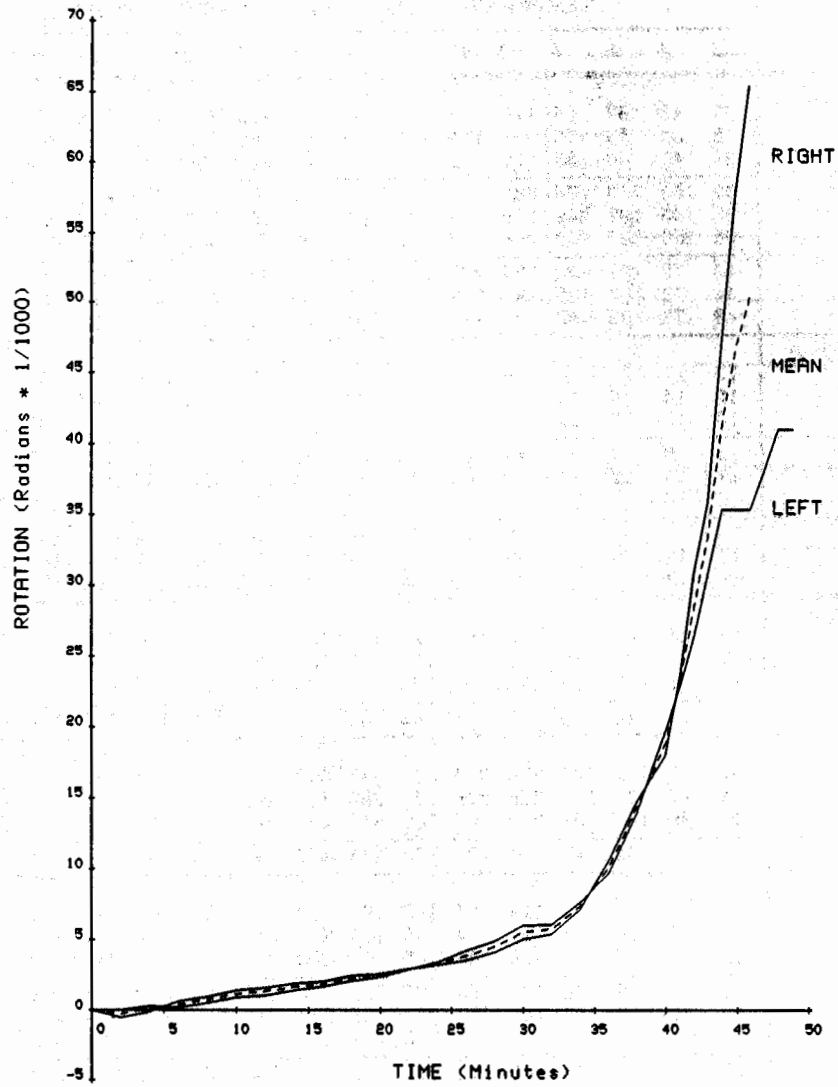
DATA SHEET NUMBER **112B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																		
	2	4	6	8	10	12	14	16	18	20	22	24	27	30	34	36	40	45	49
<b>Beam @ 25 mm from End Plate</b>																			
Upper Flange F2	37	48	67	94	126	149	177	203	232	259	287	312	352	377	414	436	474	516	546
F3	18	33	53	72	92	110	125	146	168	190	213	236	273	299	333	351	384	425	454
Mean	28	41	60	83	109	130	151	175	200	225	250	274	313	338	374	394	429	471	500
Web W2	97	155	216	276	333	379	426	472	517	555	588	615	659	683	715	735	768	793	814
W3	93	145	197	249	299	334	378	421	462	501	530	561	607	630	668	693	721	746	766
Mean	95	150	207	263	316	357	402	447	490	528	559	588	633	657	692	714	745	770	790
Lower Flange F6	42	73	107	144	187	228	270	311	353	393	433	467	525	562	615	642	685	718	737
F7	44	72	104	139	181	217	258	297	339	379	418	456	513	556	610	636	676	711	725
Mean	43	73	106	142	184	223	264	304	346	386	426	462	519	559	613	639	681	715	731
<b>Beam @ 500 mm from End Plate</b>																			
Upper Flange F1	13	17	20	24	29	35	41	48	55	62	69	76	87	97	109	115	126	145	161
F4	12	14	16	19	23	27	32	38	43	47	52	58	67	73	83	88	97	107	115
Mean	13	16	18	22	26	31	37	43	49	55	61	67	77	85	96	102	112	126	138
Web W1	13	16	23	32	45	60	73	84	98	115	131	148	175	197	229	246	278	317	347
W4	11	15	22	32	45	59	71	81	94	109	124	140	167	187	219	236	267	307	335
Mean	12	16	23	32	45	60	72	83	96	112	128	144	171	192	224	241	273	312	341
Lower Flange F5	10	12	17	24	35	47	61	74	87	103	120	138	169	195	233	254	295	344	383
F8	12	14	20	28	38	51	64	77	85	100	115	132	162	186	224	244	283	333	370
Mean	11	13	19	26	37	49	63	76	86	102	118	135	166	191	229	249	289	339	377
<b>Column / End Plate Connection</b>																			
Plate / Flange P1	16	26	40	57	83	107	127	152	181	212	244	276	330	371	428	459	515	574	616
P2	17	26	37	52	72	102	117	137	159	183	208	235	280	318	372	402	461	530	573
Mean	17	26	39	55	78	105	122	145	170	198	226	256	305	345	400	431	488	552	595
Web W5	13	14	16	20	26	35	45	56	69	84	102	112	134	152	179	195	226	269	303
<b>Column @ 300 mm from Base</b>																			
Flanges F9	51	97	151	207	276	333	391	448	502	552	595	633	687	721	756	781	823	858	881
F10	44	82	131	174	228	278	329	380	436	490	543	590	655	700	742	763	802	838	863
Mean	48	90	141	191	252	306	360	414	469	521	569	612	671	711	749	772	813	848	872
Web W6	15	20	31	47	74	100	120	144	167	190	213	236	274	303	347	369	405	448	480
<b>Exposed Bolts</b>																			
Upper B1	27	43	63	87	114	135	166	192	220	248	273	299	340	370	411	434	475	520	552
B2	29	46	69	94	121	144	170	194	219	246	272	297	340	371	416	439	480	524	555
Mean	28	45	66	91	118	140	168	193	220	247	273	298	340	371	414	437	478	522	554
Lower B3	30	48	72	100	133	163	195	228	263	298	334	368	423	463	517	544	592	643	677
B4	29	49	77	109	145	177	211	245	280	315	348	382	435	476	531	559	607	656	689
Mean	30	49	75	105	139	170	203	237	272	307	341	375	429	470	524	552	600	650	683
<b>Unexposed Bolts</b>																			
B5	16	22	34	46	60	74	91	105	124	144	161	176	205	230	262	280	313	354	387
B6	16	22	37	51	65	79	98	110	127	147	166	184	211	232	264	281	314	354	386
Mean	16	22	36	49	63	77	95	108	126	146	164	180	208	231	263	281	314	354	387
<b>Mean Furnace Gas Standard Curve</b>																			
	475	537	614	678	701	717	745	763	784	791	799	813	843	858	880	899	909	916	932
	445	544	603	645	678	705	728	748	766	781	796	809	826	842	860	869	885	902	915
<b>Rotation (Radians <math>\times 10^{-3}</math>)</b>																			
Transducers T1	0	0.3	0.2	0.5	0.9	1.1	1.4	1.7	2.1	2.3	2.9	3.4	*	6.0	7.7	9.8	19.7	35.4	41.1
T2	-0.5	-0.1	0.6	1.0	1.4	1.6	1.9	2.1	2.4	2.6	3.0	3.2	*	5.0	7.1	10.6	18.0	57.8	*
Mean	-0.3	0.1	0.4	0.8	1.2	1.4	1.7	1.9	2.3	2.5	3.0	3.3	*	5.5	7.4	10.2	18.9	46.6	*

DATA  
SHEET  
NUMBER

112C

## BEAM / COLUMN (MAJOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**113A**

BEAM / COLUMN (MAJOR AXIS) CONNECTION

DIMENSIONS AND PROPERTIES

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
203 × 203 Column	Nominal Actual	52	206.2	203.9	8.0	12.5	510.4	173.6	568.1	263.7	5263	1770
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column Beam	Grade 43A	0.23	0.04	0.90	0.024	0.019	0.04	<0.005	0.03	<0.005	0.08	<0.005	<0.005	0.0048
	Grade 43A	0.23	0.03	0.87	0.008	0.026	<0.02	<0.005	<0.02	<0.005	<0.02	<0.005	<0.005	0.0075

ROOM TEMPERATURE TENSILE PROPERTIES

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)
Column - Flange	288	501	31
Beam - Flange (Actual)	*	*	*
Beam - Flange (Mean)	295	467	34

TEST CONDITIONS

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NOTES

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TEST CENTRE : Warrington Research  
 TEST DATE : 7th December 1988  
 TEST NUMBER : WFCR 44521

## ASSESSMENT

LOAD BEARING CAPACITY  
 29 minutes at 0.2 M<sub>p</sub>

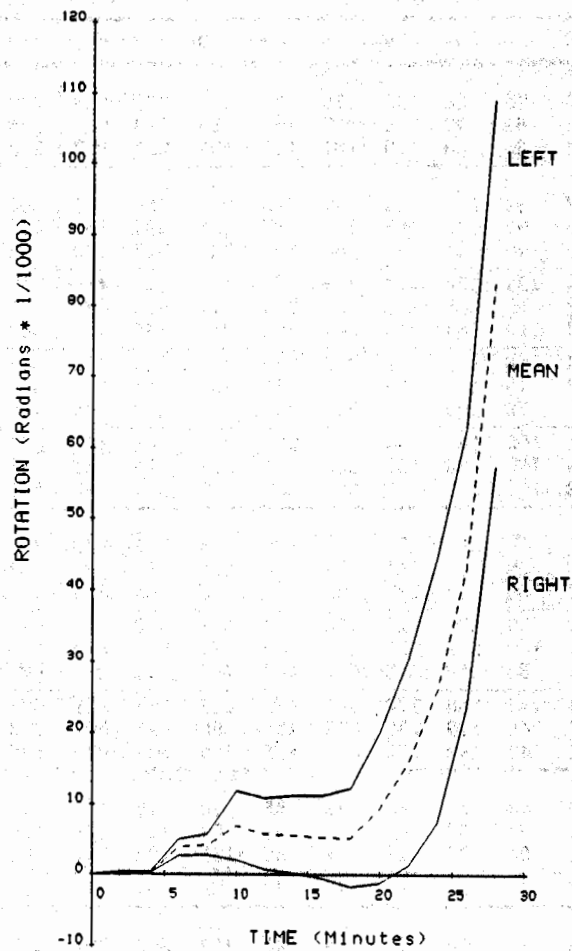
DATA SHEET  
 NUMBER **113B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)														
	2	3	4	6	8	9	10	12	15	18	21	24	27	28	29
<b>Beam @ 25 mm from End Plate</b>															
Upper Flange F2	60	75	94	135	175	190	207	250	328	400	457	502	531	540	552
F3	48	61	77	120	158	167	185	219	285	345	398	444	475	483	487
Mean	54	68	86	128	167	179	196	235	307	373	428	473	503	512	520
<b>Web</b>															
10 mm from UF W7	67	88	114	154	197	215	233	268	333	396	449	497	537	551	560
30 mm from UF W8	72	95	123	169	216	237	256	293	361	427	482	529	567	579	588
50 mm from UF W9	85	110	141	191	240	262	283	319	389	454	509	552	589	602	611
<b>Mid Height</b>															
W2	97	123	158	214	269	298	320	374	459	527	578	613	641	653	662
W3	89	115	148	205	261	286	311	355	427	491	544	585	613	622	630
Mean	93	119	153	210	265	292	316	365	443	509	561	599	627	638	646
<b>50 mm from LF</b>															
W10	64	86	115	169	222	250	279	329	402	467	521	565	598	610	622
30 mm from LF W11	59	78	104	155	207	236	265	314	387	451	505	551	584	599	610
10 mm from LF W12	60	79	105	157	212	241	270	320	394	459	513	559	595	608	619
<b>Lower Flange</b>															
F6	81	105	138	194	252	283	310	366	453	525	585	633	671	683	694
F7	83	106	138	198	262	291	324	375	451	516	569	613	648	659	670
Mean	82	106	138	196	257	287	317	371	452	521	577	623	660	671	682
<b>Beam @ 500 mm from End Plate</b>															
Upper Flange F1	57	73	91	127	166	188	209	254	332	408	476	534	581	596	611
F4	46	58	73	118	159	180	201	236	298	363	424	481	528	545	559
Mean	52	66	82	123	163	184	205	245	315	386	450	508	555	571	585
<b>Web</b>															
W1	129	173	226	321	412	453	488	552	631	686	724	743	768	779	789
W4	117	160	211	309	401	443	479	529	590	640	681	712	734	738	742
Mean	123	167	219	315	407	448	484	541	611	663	703	728	751	759	766
<b>Lower Flange</b>															
F5	104	143	190	274	361	405	444	517	611	678	722	743	770	780	790
F8	102	140	189	283	378	424	465	525	598	652	692	725	738	747	757
Mean	103	142	190	279	370	415	455	521	605	665	707	734	754	764	774
<b>Column / End Plate Connection</b>															
Plate / Flange P1	20	24	32	48	72	85	99	125	165	212	267	330	383	400	417
P2	20	24	32	47	71	85	103	123	156	198	245	300	357	376	394
Mean	20	24	32	48	72	85	101	124	161	205	256	315	370	388	406
<b>Web</b>															
W5	12	12	13	14	19	23	28	37	52	74	101	120	141	150	158
<b>Column @ 300 mm from Base</b>															
Flanges F9	56	74	100	149	199	227	253	305	392	475	547	608	658	673	687
F10	58	80	112	177	245	278	310	372	458	531	587	639	680	691	702
Mean	57	77	106	163	222	253	282	339	425	503	567	624	669	682	695
<b>Web</b>															
W6	14	15	20	30	47	59	72	103	132	163	195	228	260	272	285
<b>Exposed Bolts</b>															
Upper B1	37	46	57	86	118	133	148	180	232	281	327	362	403	417	430
B2	34	42	53	76	103	117	129	158	204	249	292	332	370	383	395
Mean	36	44	55	81	111	125	139	169	218	265	310	347	387	400	413
Lower B3	40	48	59	87	120	135	151	185	238	293	346	399	448	465	481
B4	32	41	52	80	117	138	156	192	245	300	352	398	441	456	470
Mean	36	45	56	84	119	137	154	189	242	297	349	399	445	461	476
<b>Mean Furnace Gas Standard Curve</b>															
	481	518	584	617	664	685	695	707	731	769	784	805	824	831	834
	445	502	544	603	645	663	678	705	739	766	789	809	826	832	837
<b>Rotation (Radians × 10<sup>-3</sup>)</b>															
Transducers T1	0.3	*	0.3	5.1	5.8	*	11.9	10.8	*	12.2	*	44.7	*	109.3	*
T2	0.3	*	0.2	2.7	2.8	*	2.0	0.7	*	-1.7	*	7.5	*	57.5	*
Mean	0.3	*	0.3	3.9	4.3	*	7.0	5.8	*	5.25	*	26.1	*	83.4	*

DATA  
SHEET  
NUMBER

113C

BEAM/COLUMN (MAJOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**114A****BEAM / COLUMN (MAJOR AXIS) CONNECTION****DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
203 × 203 Column	Nominal Actual	52	206.2	203.9	8.0	12.5	510.4	173.6	568.1	263.7	5263	1770
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column Beam	Grade 43A	0.23	0.04	0.90	0.024	0.019	0.04	<0.005	0.03	<0.005	0.08	<0.005	<0.005	0.0048
	Grade 43A	0.23	0.03	0.85	0.008	0.024	<0.02	<0.005	<0.02	<0.005	<0.02	<0.005	<0.005	0.0072

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)
Column - Flange	288	501	31
Beam - Flange (Actual)	291	457	33
Beam - Flange (Mean)	295	467	34

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 25th January 1989  
 TEST NUMBER : WFRC 44992

ASSESSMENT  
 LOAD BEARING CAPACITY  
 97 minutes at 0.2 M<sub>p</sub>

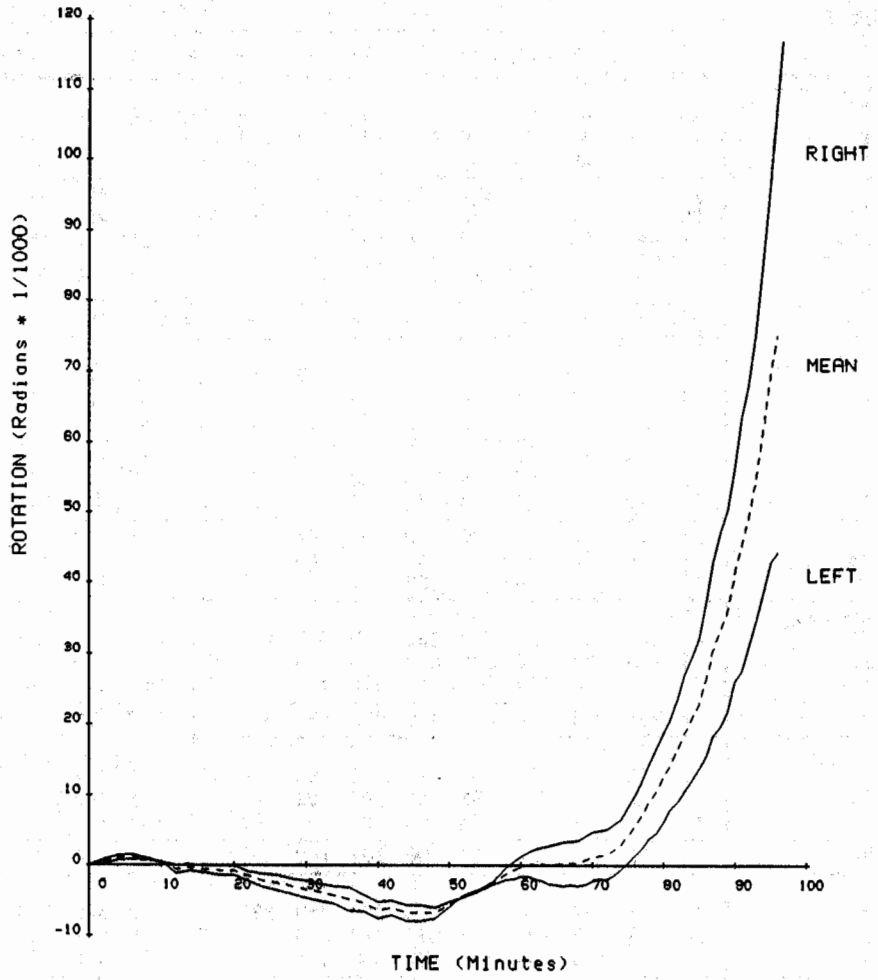
DATA SHEET NUMBER **114B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																		
	3	6	9	12	15	18	21	24	27	30	35	40	50	60	70	80	90	97	
<u>Beam @ 25 mm from End Plate</u>																			
Upper Flange	F2	18	31	49	72	97	116	140	167	189	210	236	266	327	377	430	478	521	551
	F3	15	30	49	73	99	120	142	164	186	209	245	280	345	397	448	498	548	589
	Mean	17	31	49	73	98	118	141	166	188	210	241	273	336	387	439	488	535	570
Web	W2	21	47	79	119	170	211	256	298	336	374	430	480	563	617	669	724	781	820
	W3	20	47	81	125	175	217	261	303	343	381	439	492	580	635	684	739	795	834
	Mean	21	47	80	122	173	214	259	301	340	378	435	486	572	626	677	732	788	827
Lower Flange	F6	18	37	60	87	125	160	198	235	269	304	358	410	501	571	635	693	741	777
	F7	18	34	57	85	121	155	192	228	262	296	349	401	493	564	626	685	733	769
	Mean	18	36	59	86	123	158	195	232	266	300	354	406	497	568	631	689	737	773
<u>Beam @ 500 mm from End Plate</u>																			
Upper Flange	F1	12	13	15	17	21	24	30	36	41	46	55	66	83	98	117	137	159	178
	F4	13	13	14	16	19	22	27	32	37	42	51	60	80	94	109	127	146	162
	Mean	13	13	15	17	20	23	29	34	39	44	53	63	82	96	113	132	153	170
Web	W1	8	10	15	21	32	41	54	66	76	85	111	140	193	238	282	333	384	420
	W4	11	13	19	25	35	46	60	73	81	90	115	141	193	238	285	336	387	423
	Mean	10	12	17	23	34	44	57	70	79	88	113	141	193	238	284	335	386	422
Lower Flange	F5	12	14	16	21	29	39	51	65	78	89	116	148	211	272	334	399	463	507
	F8	8	8	13	18	27	37	51	65	77	92	119	149	211	272	333	399	464	508
	Mean	10	11	15	20	28	38	51	65	78	91	118	149	211	272	334	399	464	508
<u>Column / End Plate Connection</u>																			
Plate / Flange	P1	55	107	166	231	299	350	400	445	485	522	580	631	708	747	820	884	931	960
	P2	59	115	173	235	300	352	404	449	491	528	587	639	719	760	830	891	938	966
	Mean	57	111	170	233	300	351	402	447	488	525	584	635	714	754	825	888	935	963
Web	W5	117	206	299	390	463	509	550	587	620	650	694	732	781	814	878	923	962	984
<u>Column @ 300 mm from Base</u>																			
Flanges	F9	11	11	14	27	57	92	98	98	104	114	141	177	258	344	423	497	564	605
	F10	9	10	12	22	51	88	99	99	103	110	133	165	248	337	420	498	566	610
	Mean	10	11	13	25	54	90	99	99	104	112	137	171	253	341	422	498	565	608
Web	W6	12	13	18	32	70	99	102	102	108	118	141	175	259	349	432	508	574	616
<u>Exposed Bolts</u>																			
Upper	B1	49	90	136	184	234	276	320	359	395	428	482	530	608	660	717	772	834	875
	B2	50	88	142	196	246	282	319	356	391	425	475	521	595	642	704	754	813	850
	Mean	50	89	139	190	240	279	320	358	393	427	479	526	602	651	711	763	824	863
Lower	B3	40	75	120	173	229	277	330	378	421	463	523	577	663	713	768	838	897	930
	B4	38	72	123	181	238	281	326	370	413	453	513	567	652	701	753	820	882	915
	Mean	39	74	122	177	234	279	328	374	417	458	518	572	658	707	761	829	890	923
<u>Unexposed Bolts</u>																			
	B5	16	31	49	71	99	108	128	146	169	193	232	267	333	394	445	502	565	630
	B6	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
	Mean	16	31	49	71	99	108	128	146	169	193	232	267	333	394	445	502	565	630
<u>Mean Furnace Gas Standard Curve</u>																			
		504	601	663	699	726	741	757	773	786	802	824	847	851	895	945	977	1006	1023
		502	603	663	705	739	766	789	809	826	842	865	885	918	945	968	988	1006	1017
<u>Rotation (Radians × 10<sup>-3</sup>)</u>																			
Transducers	T1	*	0.8	*	0.0	*	-0.1	*	-1.3	*	-2.3	*	-5.3	-5.3	-1.6	-2.3	6.1	26.1	*
	T2	*	1.6	*	-1.2	*	-1.4	*	-3.2	*	-4.8	*	-7.5	-6.0	1.2	4.8	18.7	56.3	117
	Mean	*	1.2	*	-0.6	*	-0.8	*	-2.3	*	-3.6	*	-6.4	-5.7	-0.2	1.3	12.4	41.2	*

DATA  
SHEET  
NUMBER

114C

BEAM / COLUMN (MAJOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**115A****BEAM / COLUMN (MAJOR AXIS) CONNECTION****DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
203 × 203 Column	Nominal Actual	52	206.2	203.9	8.0	12.5	510.4	173.6	568.1	263.7	5263	1770
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column	Grade 43A	0.23	0.04	0.90	0.024	0.019	0.04	<0.005	0.03	<0.005	0.08	<0.005	<0.005	0.0048
Beam	Grade 43A	0.23	0.03	0.87	0.008	0.026	<0.02	<0.005	<0.02	<0.005	<0.02	<0.005	<0.005	0.0075

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)
Column - Flange	288	501	31
Beam - Flange (Actual)	*	*	*
Beam - Flange (Mean)	295	467	34

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 7th December 1988  
 TEST NUMBER : WFRC 44521

## ASSESSMENT

LOAD BEARING CAPACITY  
 122 minutes at 0.2 M<sub>p</sub>

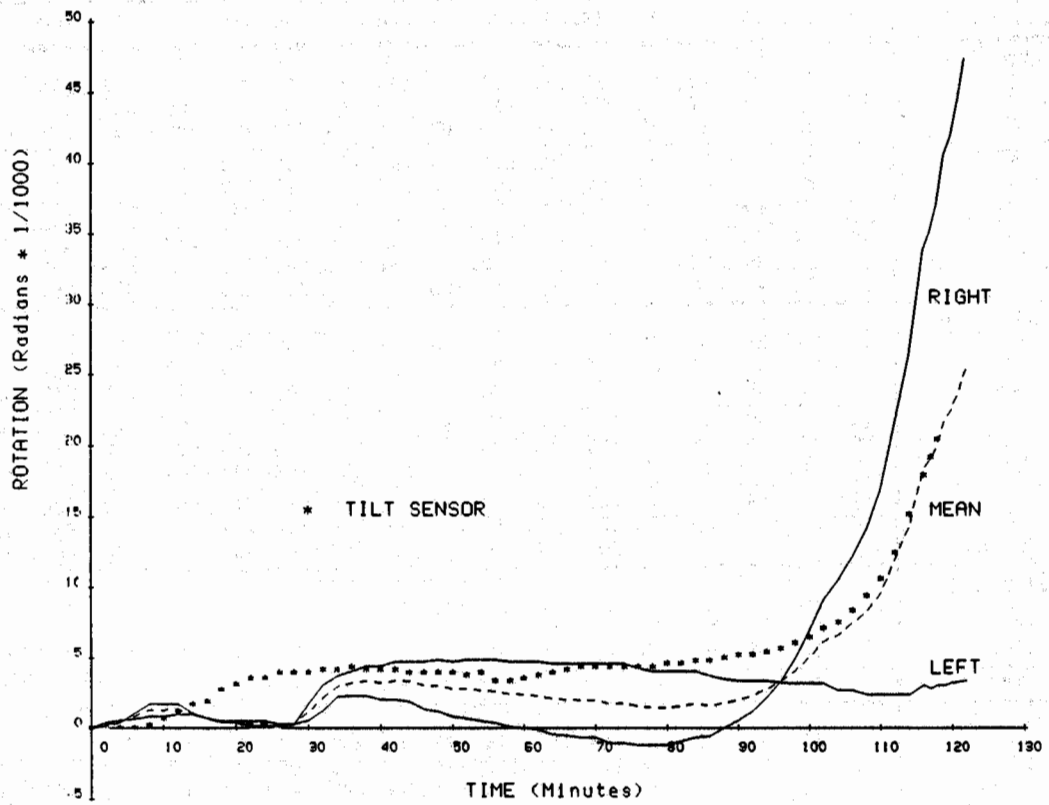
DATA SHEET  
 NUMBER **115B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																		
	3	6	9	12	15	18	21	24	27	30	40	50	60	70	80	90	100	110	122
<b>Beam @ 25 mm from End Plate</b>																			
Upper Flange F2	14	20	30	40	52	67	83	93	99	100	117	156	210	266	313	354	392	432	479
F3	17	25	35	47	61	77	91	104	107	109	149	192	247	300	346	387	430	472	519
Mean	16	23	33	44	57	72	87	99	103	105	133	174	229	283	330	371	411	452	499
Web W2	14	20	35	52	71	94	103	105	104	105	142	212	282	349	409	464	512	554	602
W3	13	18	33	50	77	99	103	103	103	105	145	217	287	355	414	469	517	561	608
Mean	14	19	34	51	74	97	103	104	104	105	144	215	285	352	412	467	515	558	605
Lower Flange F6	17	28	45	62	80	99	105	110	119	132	195	267	343	417	483	542	593	637	685
F7	17	31	49	66	84	98	107	119	133	151	218	294	371	445	509	566	616	662	710
Mean	17	30	47	64	82	99	106	115	126	142	207	281	357	431	496	554	605	650	698
<b>Beam @ 500 mm from End Plate</b>																			
Upper Flange F1	15	24	34	43	53	64	76	88	96	102	118	148	184	221	257	291	324	355	391
F4	16	27	37	47	60	76	90	99	106	111	135	167	209	252	292	329	363	396	434
Mean	16	26	36	45	57	70	83	94	101	107	127	158	197	237	275	310	344	376	413
Web W1	16	37	69	92	103	107	110	117	137	163	257	340	415	484	544	596	639	678	718
W4	18	43	73	95	103	109	119	146	178	210	304	386	458	525	582	631	673	711	742
Mean	17	40	71	94	103	108	115	132	158	187	281	363	437	505	563	614	656	695	730
Lower Flange F5	17	35	58	78	97	104	104	115	133	155	236	331	427	510	580	639	688	728	755
F8	18	40	67	89	102	106	119	145	172	200	296	394	484	564	630	685	729	753	796
Mean	18	38	63	84	100	105	112	130	153	178	266	363	456	537	605	662	709	741	776
<b>Column / End Plate Connection</b>																			
Plate / Flange P1	18	30	48	66	85	102	115	127	141	*	*	*	*	*	*	*	*	*	*
P2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Mean	18	30	48	66	85	102	115	127	141	*	*	*	*	*	*	*	*	*	*
Web W5	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
<b>Column @ 300 mm from Base</b>																			
Flanges F9	16	37	61	83	100	108	128	154	177	205	293	381	*	*	*	*	*	*	*
F10	17	39	63	85	102	111	134	161	185	210	304	395	477	550	611	664	709	740	771
Mean	17	38	62	84	101	110	131	158	181	208	299	388	477	550	611	664	709	740	771
Web W6	16	34	61	85	102	108	114	136	162	192	294	388	472	546	608	661	705	741	773
<b>Exposed Bolts</b>																			
Upper B1	17	26	38	49	64	80	94	106	113	*	156	198	249	304	356	403	447	491	*
B2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Mean	17	26	38	49	64	80	94	106	113	*	156	198	249	304	356	403	447	491	*
Lower B3	19	24	37	53	70	91	106	*	*	*	188	254	321	387	448	507	561	613	*
B4	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Mean	19	24	37	53	70	91	106	*	*	*	188	254	321	387	448	507	561	613	*
<b>Mean Furnace Gas Standard Curve</b>	518	617	685	707	731	769	784	805	824	848	890	934	970	996	1007	1025	1040	1061	1082
	502	603	663	705	739	766	789	809	826	842	885	918	945	968	988	1006	1022	1036	1052
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																			
Transducers T1	*	1.0	*	1.7	*	0.3	*	0.1	*	1.7	4.4	4.7	4.7	4.6	4.1	3.4	3.2	2.4	3.4
T2	*	0.6	*	0.9	*	0.5	*	0.5	*	0.5	2.0	0.8	0.0	-0.7	-1.2	0.5	7.0	16.9	47.4
Mean	*	0.8	*	1.3	*	0.4	*	0.3	*	1.1	3.2	2.8	2.4	2.0	1.5	2.0	5.1	9.7	25.4
Tilt Sensor	*	0	*	1.0	*	2.7	*	3.6	*	4.0	4.2	4.0	3.6	4.4	4.6	5.2	6.5	10.7	*

DATA  
SHEET  
NUMBER

115C

BEAM / COLUMN (MAJOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**116A****BEAM / COLUMN (MAJOR AXIS) CONNECTION****DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
203 × 203 Column	Nominal Actual	52	206.2	203.9	8.0	12.5	510.4	173.6	568.1	263.7	5263	1770
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column Beam	Grade 43A	0.23	0.04	0.90	0.024	0.019	0.04	<0.005	0.03	<0.005	0.08	<0.005	<0.005	0.0048
	Grade 43A	0.23	0.03	0.89	0.008	0.026	<0.02	<0.005	<0.02	<0.005	<0.02	<0.005	<0.005	0.0076

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)
Column - Flange	288	501	31
Beam - Flange (Actual)	*	*	*
Beam - Flange (Mean)	295	467	34

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 11th January 1989  
 TEST NUMBER : WFRC 44519

## ASSESSMENT

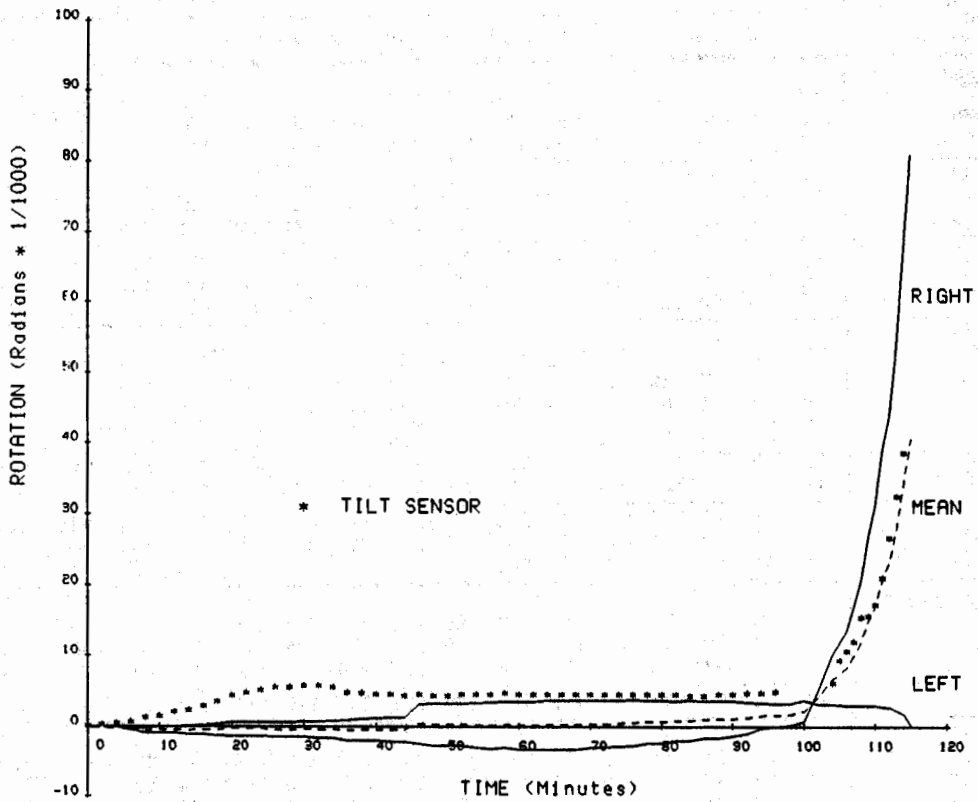
LOAD BEARING CAPACITY  
 115 minutes at 0.1 M<sub>p</sub>

DATA SHEET NUMBER **116B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																			
	2	6	12	18	24	28	30	36	40	46	50	56	60	70	80	90	100	110	115	
<b>Beam @ 25 mm from End</b>																				
Upper Flange	F2	10	27	54	83	106	118	125	143	156	185	206	240	262	313	357	398	436	474	491
	F3	9	16	43	76	97	99	100	110	130	160	182	216	239	290	337	381	421	473	505
	Mean	10	22	49	80	102	109	113	127	143	173	194	228	251	302	347	390	429	474	498
Web	W2	11	17	50	86	102	105	109	142	172	217	245	287	314	379	439	493	542	586	606
	W3	10	15	48	84	100	102	102	123	157	212	245	292	323	393	457	515	566	612	633
	Mean	11	16	49	85	101	104	106	133	165	215	245	290	319	386	448	504	554	599	620
Lower Flange	F6	10	31	73	106	132	158	173	222	256	310	345	396	428	500	563	621	672	719	737
	F7	10	21	65	97	110	133	146	194	227	280	316	369	403	481	549	609	662	706	724
	Mean	10	26	69	102	121	146	160	208	242	295	331	383	416	491	556	615	667	713	731
<b>Beam @ 500 mm from End</b>																				
Upper Flange	F1	9	26	45	65	87	96	100	120	140	166	181	204	220	258	294	330	362	393	407
	F4	9	17	37	59	80	91	94	101	108	128	142	164	180	220	259	296	331	362	377
	Mean	9	22	41	62	84	94	97	111	124	147	162	184	200	239	277	313	347	378	392
Web	W1	11	38	96	112	153	194	216	277	315	367	399	443	472	534	587	634	675	707	720
	W4	10	45	94	134	166	202	221	281	319	372	405	451	480	543	598	646	687	722	737
	Mean	11	42	95	123	160	198	219	279	317	370	402	447	476	539	593	640	681	715	729
Lower Flange	F5	11	35	87	108	153	191	210	272	314	377	416	470	504	578	639	691	733	755	770
	F8	10	32	83	104	139	177	195	251	292	352	390	444	479	556	620	674	721	746	760
	Mean	11	34	85	106	146	184	203	262	303	365	403	457	492	567	630	683	727	751	765
<b>Column / Web Cleat Connection</b>																				
Cleat / Flange	C1	12	30	64	97	127	147	157	190	215	252	277	316	342	404	461	516	564	608	628
	C2	14	33	70	105	137	161	173	212	239	279	306	347	375	440	501	559	608	653	674
	Mean	13	32	67	101	132	154	165	201	227	266	292	332	359	422	481	538	586	631	651
Web	W5	10	34	80	100	136	166	180	223	251	293	320	361	389	452	512	567	618	661	681
<b>Column @ 300 mm from Base</b>																				
Flanges	F9	10	38	94	124	180	221	243	310	353	416	453	507	540	612	672	723	755	785	803
	F10	11	34	87	106	152	195	218	287	333	397	436	490	524	598	659	710	745	772	791
	Mean	11	36	91	115	166	208	231	299	343	407	445	499	532	605	666	717	750	779	797
Web	W6	13	33	86	104	140	190	215	286	332	397	435	490	523	596	655	706	743	769	786
<b>Exposed Bolts</b>																				
Upper	B1	12	26	58	87	108	118	127	155	175	208	231	267	292	351	407	461	511	555	575
	B2	11	26	63	95	115	133	142	175	197	233	258	298	324	386	445	504	557	607	634
	Mean	12	26	61	91	112	126	135	165	186	221	245	283	308	369	426	483	534	581	605
Lower	B3	13	22	57	93	112	122	133	174	204	252	284	330	362	433	498	556	607	650	670
	B4	12	26	64	97	121	138	149	193	224	273	305	354	385	457	523	582	633	675	694
	Mean	13	24	61	95	117	130	141	184	214	263	295	342	374	445	511	569	620	663	682
Web Cleat	B5	10	19	47	80	101	108	111	131	152	192	218	258	285	347	404	457	506	551	575
	B6	10	15	43	82	100	103	108	135	162	211	242	291	323	396	462	521	573	618	639
<b>Mean Furnace Gas Standard Curve</b>		471	609	723	777	818	842	855	881	904	926	941	964	970	998	1017	1045	1043	1059	1061
		445	603	705	766	809	832	842	869	885	906	918	935	945	968	988	1006	1022	1036	1043
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																				
Transducers	T1	-0.2	-0.4	0.0	0.3	0.6	0.6	0.7	0.9	1.2	3.1	3.2	3.4	3.5	3.7	3.7	3.6	3.7	2.9	0.0
	T2	0.0	-0.5	-0.9	-1.3	-1.4	-1.5	-1.5	-2.0	-2.0	-2.5	-2.7	-3.2	-3.3	-3.0	-2.4	-1.4	0.7	31.1	81.1
	Mean	-0.1	-0.5	-0.5	-0.5	-0.4	-0.5	-0.4	-0.6	-0.4	0.3	0.3	0.1	0.1	0.4	0.7	1.1	2.2	17.0	40.6
Tilt Sensor		0.2	0.6	2.0	3.5	5.1	5.5	5.7	4.7	4.5	4.5	4.3	4.5	4.5	4.5	4.5	4.5	*	17.3	*

DATA SHEET NUMBER **116C**

BEAM / COLUMN (MAJOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**117A****BEAM / COLUMN (MAJOR AXIS) CONNECTION****DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
203 × 203 Column	Nominal Actual	52	206.2	203.9	8.0	12.5	510.4	173.6	568.1	263.7	5263	1770
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column Beam	Grade 43A	0.23	0.04	0.90	0.024	0.019	0.04	<0.005	0.03	<0.005	0.08	<0.005	<0.005	0.0048
	Grade 43A	0.23	0.03	0.87	0.008	0.026	<0.02	<0.005	<0.02	<0.005	<0.02	<0.005	<0.005	0.0075

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)
Column - Flange	288	501	31
Beam - Flange (Actual)	*	*	*
Beam - Flange (Mean)	295	467	34

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 11th January 1989  
 TEST NUMBER : WFRC 44519

## ASSESSMENT

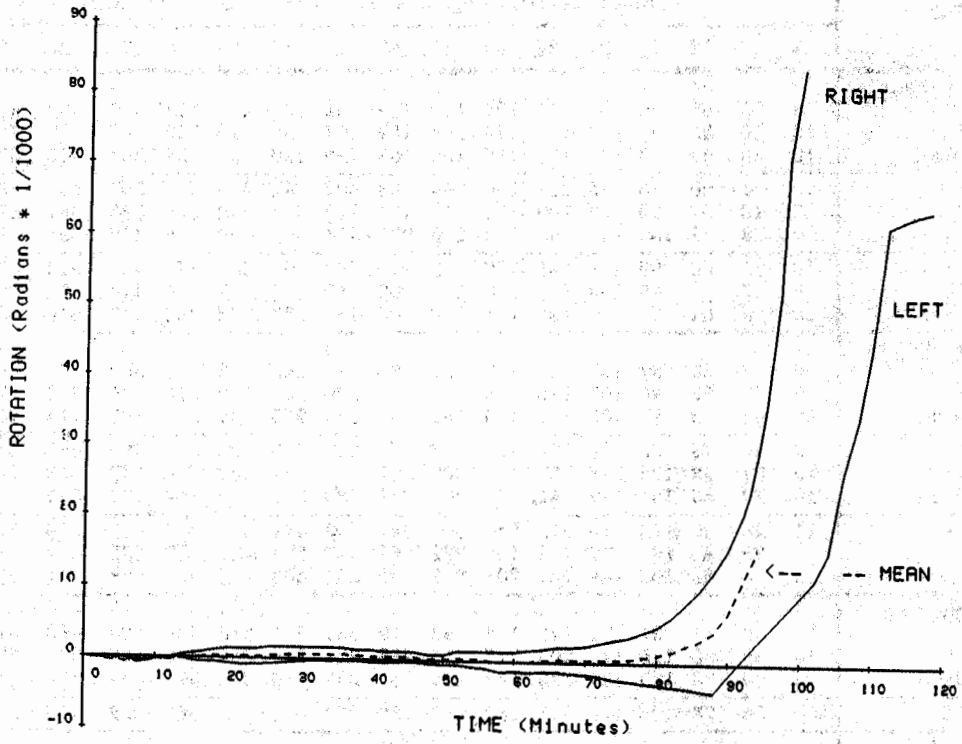
LOAD BEARING CAPACITY  
 100 minutes at 0.4 M<sub>p</sub>  
 123 minutes at 0.3 M<sub>p</sub>

DATA SHEET NUMBER **117B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																		
	2	6	12	18	24	28	30	36	40	46	50	56	60	70	80	90	100	110	123
<b>Beam @ 25 mm from End Plate</b>																			
Upper Flange																			
F2	9	20	52	79	100	104	107	129	149	183	207	246	273	340	400	456	512	577	654
F3	10	20	53	87	107	116	122	148	169	207	233	274	302	368	429	488	551	621	697
Mean	10	20	53	83	104	110	115	139	159	195	220	260	288	354	415	472	532	599	676
Web																			
W2	10	41	100	115	165	213	236	304	348	407	444	495	527	597	655	704	745	774	821
W3	8	15	58	99	100	100	104	141	174	222	253	298	328	398	461	519	568	621	688
Mean	9	28	79	107	133	157	170	223	261	315	349	397	428	498	558	612	657	698	755
Lower Flange																			
F6	10	24	63	99	109	125	136	179	210	258	290	338	371	445	512	570	622	666	713
F7	9	22	65	99	103	116	126	172	203	252	284	333	365	440	508	567	620	667	717
Mean	10	23	64	99	106	121	131	176	207	255	287	336	368	443	510	569	621	667	715
<b>Beam @ 500 mm from End Plate</b>																			
Upper Flange																			
F1	9	20	63	97	102	113	122	157	184	222	246	282	306	359	408	454	498	535	579
F4	10	23	62	96	107	120	128	164	189	229	254	292	318	375	427	476	521	562	608
Mean	10	22	63	97	105	117	125	161	187	226	250	287	312	367	418	465	510	549	594
Web																			
W1	9	20	73	98	98	100	105	140	173	222	253	300	331	398	460	517	566	612	669
W4	10	37	93	104	141	181	202	262	301	358	393	444	476	548	608	662	708	745	797
Mean	10	29	83	101	120	141	154	201	237	290	323	372	404	473	534	590	637	679	733
Lower Flange																			
F5	11	46	93	115	179	219	240	302	342	403	441	494	527	600	660	712	754	784	830
F8	9	29	83	101	110	143	162	222	262	325	364	421	457	535	602	659	707	738	775
Mean	10	38	88	108	145	181	201	262	302	364	403	458	492	568	631	686	731	761	803
<b>Column / End Plate Connection</b>																			
Plate / Flange																			
P1	11	30	61	91	121	142	152	193	219	257	281	318	344	403	460	515	557	608	674
P2	14	49	85	115	177	203	217	255	282	321	345	381	404	465	521	573	618	670	735
Mean	13	40	73	103	149	173	185	224	251	289	313	350	374	434	491	544	588	639	705
Web																			
W5	10	34	74	100	119	148	163	209	237	279	305	343	369	429	486	541	587	633	692
<b>Column @ 300 mm from Base</b>																			
Flanges																			
F9	11	40	91	111	170	216	239	309	352	414	452	504	537	608	666	716	747	777	832
F10	10	32	87	102	145	185	207	272	316	380	418	472	507	580	642	694	735	756	807
Mean	11	36	89	107	158	201	223	291	334	397	435	488	522	594	654	705	741	767	820
Web																			
W6	10	34	88	104	149	197	221	291	336	399	437	491	524	597	656	707	743	771	819
<b>Exposed Bolts</b>																			
Upper																			
B1	10	20	50	82	105	115	121	142	160	192	215	252	277	339	398	453	508	579	672
B2	10	19	48	80	100	110	117	145	164	199	223	260	287	348	407	462	523	624	718
Mean	10	20	49	81	103	113	119	144	162	196	219	256	282	344	403	458	516	602	695
Lower																			
B3	10	21	56	94	100	116	128	167	198	244	274	319	349	419	486	548	600	646	701
B4	10	20	52	88	106	116	125	159	187	232	261	306	336	408	476	542	593	636	700
Mean	10	21	54	91	103	116	127	163	193	238	268	313	343	414	481	545	597	641	701
<b>Mean Furnace Gas Standard Curve</b>																			
	471	609	723	777	818	842	855	881	904	926	941	964	970	998	1017	1045	1043	1059	1073
	445	603	705	766	809	832	842	869	885	906	918	935	945	968	988	1006	1022	1036	1053
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																			
Transducers																			
T1	-0.3	-0.7	-0.4	-0.6	-0.8	-0.6	-0.4	-0.2	-0.4	-0.3	-0.6	-1.0	-1.4	-1.7	-2.9	-1.7	*	45.3	*
T2	0.0	-0.3	0.0	1.0	1.3	1.6	1.6	1.6	1.3	1.2	0.8	1.3	1.3	2.3	5.2	16.3	84.5	*	*
Mean	-0.2	-0.5	-0.2	0.2	0.3	0.5	0.6	0.7	0.5	0.5	0.1	0.2	-0.1	0.3	1.2	7.3	*	*	*

DATA SHEET NUMBER **117C**

BEAM/COLUMN (MAJOR AXIS) CONNECTION



DATA  
SHEET  
NUMBER

**118A**

**BEAM / COLUMN (MAJOR AXIS) CONNECTION**

**DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
203 × 203 Column	Nominal Actual	52	206.2	203.9	8.0	12.5	510.4	173.6	568.1	263.7	5263	1770
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column Beam	Grade 43A	0.23	0.04	0.90	0.024	0.019	0.04	<0.005	0.03	<0.005	0.08	<0.005	<0.005	0.0048
	Grade 43A	0.22	0.03	0.88	0.008	0.027	<0.02	<0.005	<0.02	<0.005	<0.02	<0.005	<0.005	0.0074

**ROOM TEMPERATURE TENSILE PROPERTIES**

**TEST CONDITIONS**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)
Column - Flange	288	501	31
Beam - Flange (Actual)	298	468	34
Beam - Flange (Mean)	295	467	34

**NOTES**

TEST CENTRE : Warrington Research  
 TEST DATE : 18th January 1989  
 TEST NUMBER : WFRC 44175

## ASSESSMENT

LOAD BEARING CAPACITY  
 111 minutes at 0.2 M<sub>p</sub>

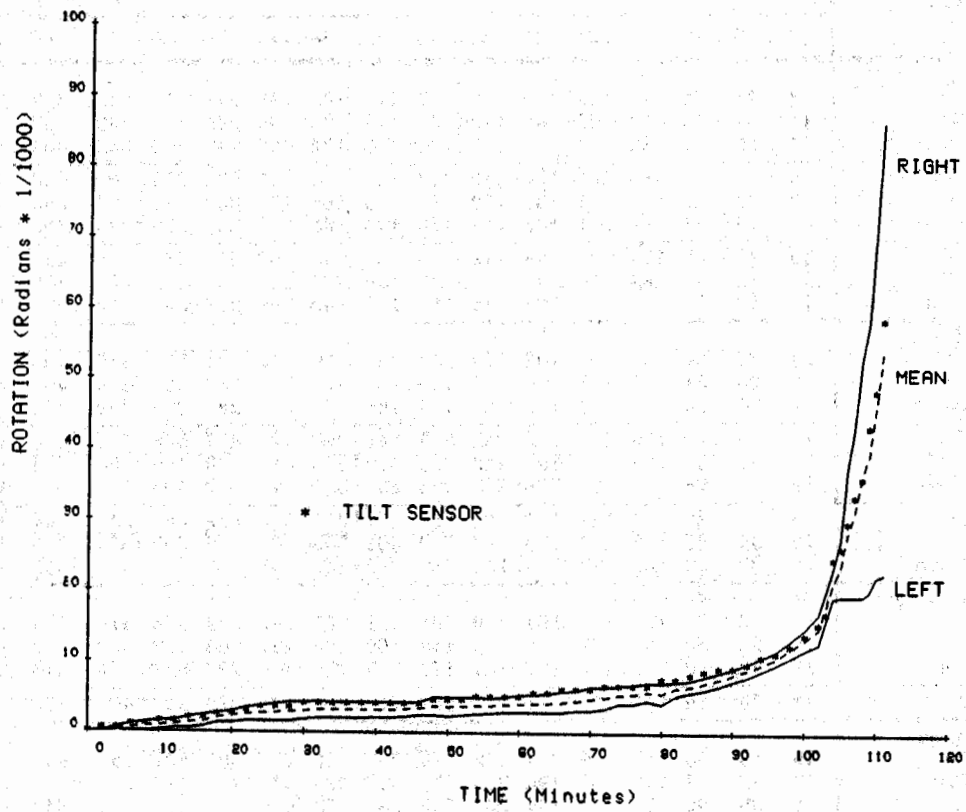
DATA SHEET NUMBER **118B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																		
	2	6	12	18	24	27	30	36	40	46	50	56	60	70	80	90	100	111	
<b>Beam @ 25 mm from End</b>																			
Upper Flange	F2	14	27	60	90	103	106	107	122	142	180	208	248	275	343	407	464	514	572
	F3	14	24	54	82	103	107	112	130	148	183	206	245	272	340	402	454	503	576
	Mean	14	26	57	86	103	107	110	126	145	182	207	247	274	342	405	459	509	574
Web	W2	14	19	47	85	101	104	110	138	163	204	231	272	299	365	426	482	533	582
	W3	13	16	40	83	101	101	102	113	129	166	193	233	260	324	385	440	491	540
	Mean	14	18	44	84	101	103	106	126	146	185	212	253	280	345	406	461	512	561
Lower Flange	F6	15	20	52	100	102	108	119	159	192	236	266	312	342	415	483	540	592	641
	F7	13	18	48	84	100	101	106	145	179	224	255	302	332	405	470	526	577	627
	Mean	14	19	50	92	101	105	113	152	186	230	261	307	337	410	477	533	585	634
<b>Beam @ 500 mm from End</b>																			
Upper Flange	F1	14	22	54	97	101	102	113	153	180	219	244	281	304	358	407	450	491	529
	F4	13	25	54	89	105	120	134	166	189	225	249	283	306	359	408	453	493	536
	Mean	14	24	54	93	103	111	124	160	185	222	247	282	305	359	408	452	492	533
Web	W1	15	46	102	116	179	215	242	303	341	396	429	477	506	572	628	675	715	742
	W4	14	40	94	109	162	197	223	283	322	378	412	460	490	556	613	661	703	735
	Mean	15	43	98	113	171	206	233	293	332	387	421	469	498	564	621	668	709	739
Lower Flange	F5	15	31	80	103	127	155	181	239	277	335	374	429	464	541	604	657	703	737
	F8	15	34	85	106	137	170	197	259	301	363	402	456	490	564	626	678	722	749
	Mean	15	33	83	105	132	163	189	249	289	349	388	443	477	553	615	668	713	743
<b>Column / Web Cleat Connection</b>																			
Cleat / Flange	C1	17	33	71	105	136	154	169	205	231	267	292	329	354	416	475	528	578	628
	C2	14	23	52	86	102	110	119	148	169	203	227	264	289	352	412	468	519	569
	Mean	16	28	62	96	119	132	144	177	200	235	260	297	322	384	444	498	549	599
Web	W5	13	30	76	101	104	123	141	187	217	256	281	319	343	405	463	517	566	614
<b>Column @ 300 mm from Base</b>																			
Flanges	F9	16	43	95	118	179	214	242	310	353	413	450	501	532	599	657	704	742	771
	F10	13	32	84	106	160	194	223	290	333	393	430	481	513	581	638	686	726	757
	Mean	15	38	90	112	170	204	233	300	343	403	440	491	523	590	648	695	734	764
Web	W6	14	36	90	104	145	188	222	297	341	404	441	492	523	591	648	695	734	762
<b>Exposed Bolts</b>																			
Upper	B1	15	23	50	82	107	111	114	130	145	171	192	224	247	307	363	414	462	522
	B2	16	29	64	97	117	129	142	176	199	232	254	289	313	371	426	477	525	574
	Mean	16	26	57	90	112	120	128	153	172	202	223	257	280	339	395	446	494	548
Lower	B3	16	21	50	85	109	112	119	148	172	211	237	278	306	372	435	494	551	604
	B4	17	31	67	104	123	137	153	194	222	264	292	333	360	426	488	543	592	642
	Mean	17	26	59	95	116	125	136	171	197	238	265	306	333	399	462	519	572	623
Web Cleat	B5	15	17	40	76	105	106	105	113	126	159	190	233	264	338	406	468	526	577
	B6	15	19	45	80	102	104	106	124	143	177	204	243	270	336	398	453	503	554
	Mean	15	18	43	78	104	105	106	119	135	168	197	238	267	337	402	461	515	566
<b>Mean Furnace Gas Standard Curve</b>		475	614	717	784	813	843	858	899	909	918	939	951	958	982	1001	1016	1036	1049
		445	603	705	766	809	826	842	869	885	906	918	935	945	968	988	1006	1022	1037
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																			
Transducers	T1	0.0	0.0	0.5	1.3	1.6	*	1.8	2.0	2.1	2.5	2.3	2.8	2.9	3.2	4.2	7.5	11.8	22.7
	T2	0.0	1.0	1.7	2.7	3.7	*	4.3	4.3	4.3	4.3	5.0	4.9	5.3	6.6	7.2	9.3	14.8	86.7
	Mean	0.0	0.5	1.1	2.0	2.7	*	3.1	3.2	3.2	3.4	3.7	3.9	4.1	4.9	5.7	8.4	13.3	54.7
Tilt Sensor		0.5	1.0	1.5	2.6	3.6	*	4.1	4.1	4.1	4.1	4.6	5.2	5.2	6.2	7.7	9.3	13.9	58.9



DATA SHEET NUMBER **118C**

BEAM / COLUMN (MAJOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**119A****SHELF ANGLE FLOOR BEAM / COLUMN (MAJOR AXIS) CONNECTION****DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
203 × 203 Column	Nominal Actual	52	206.2	203.9	8.0	12.5	510.4	173.6	568.1	263.7	5263	1770
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763
125 × 75 Angle	Nominal Actual	17.8	125	75	12	12	43.2	16.9	77.36	31.42	354	95.5

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column	Grade 43A	0.23	0.04	0.90	0.024	0.019	0.04	<0.005	0.03	<0.005	0.08	<0.005	<0.005	0.0048
Beam	Grade 43A	0.24	0.03	0.87	0.008	0.026	<0.02	<0.005	<0.02	<0.005	<0.02	<0.005	<0.005	0.0077
Angle	Grade 43A	0.15	0.02	1.35	0.010	0.019	0.02	<0.005	0.02	<0.001	0.03	<0.005	<0.005	0.0056

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)
Column - Flange	288	501	31
Beam - Flange (Actual)	286	467	36
Beam - Flange (Mean)	295	467	34
Angle	277	473	34

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 25th January 1989  
 TEST NUMBER : WFRS 44992

## ASSESSMENT

LOAD BEARING CAPACITY  
 70 minutes at 0.2 M<sub>p</sub>  
 90 minutes at 0.1 M<sub>p</sub>

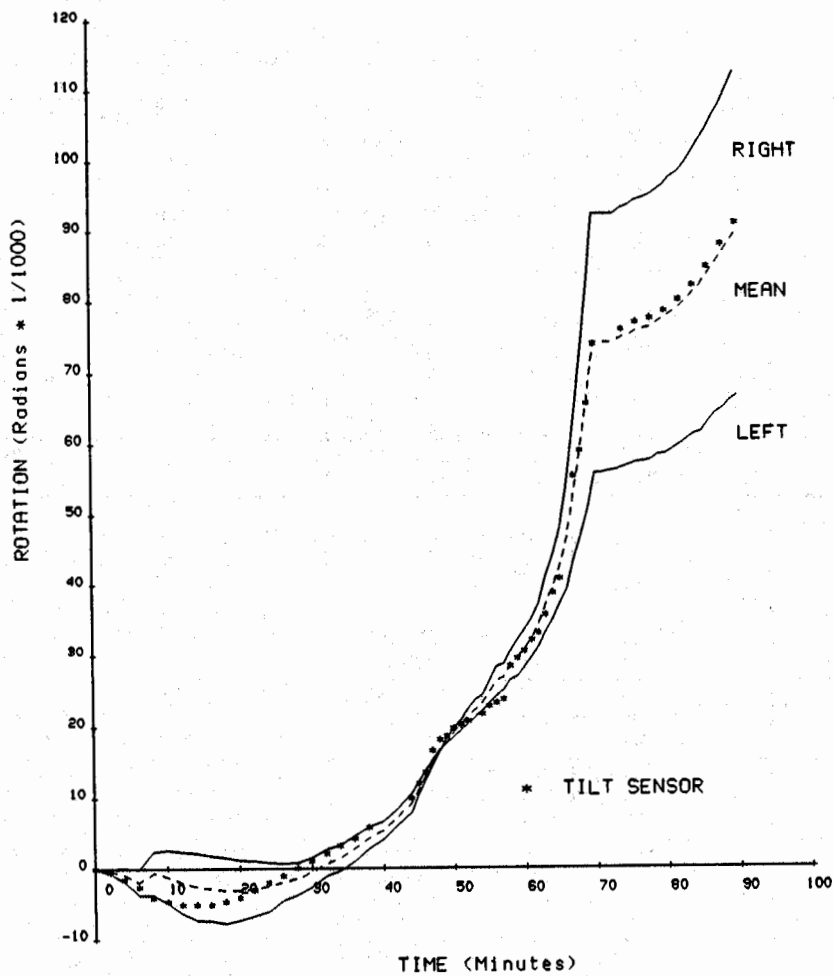
DATA SHEET  
 NUMBER **119B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																		
	3	6	9	12	15	18	21	24	27	30	35	40	45	50	55	60	70	80	90
<b>Beam @ 25 mm from End Plate</b>																			
Upper Flange F2	12	13	17	24	34	43	56	69	82	91	108	126	146	165	184	202	240	283	327
F3	11	15	23	43	71	88	100	102	102	110	129	150	171	191	209	229	268	310	351
Mean	12	14	20	34	53	66	78	86	92	101	119	138	159	178	197	216	254	297	339
Unexposed Web W2	13	18	28	41	58	73	89	106	119	132	160	187	214	238	261	284	328	375	421
W3	15	22	36	58	78	95	104	114	134	151	178	203	228	253	275	294	339	383	429
Mean	14	20	32	50	68	84	97	110	127	142	169	195	221	246	268	289	334	379	425
Unexposed Angle A2	20	34	55	83	107	131	158	187	215	241	282	323	362	395	422	445	501	555	604
A3	20	39	92	102	104	125	160	193	224	252	291	326	358	389	414	436	488	544	592
Mean	20	37	74	93	106	128	159	190	220	247	287	325	360	392	418	441	495	550	598
Exposed Angle A6	38	66	113	155	201	232	267	301	333	367	419	466	513	549	574	602	670	733	790
A7	52	98	138	170	225	272	315	354	389	421	470	513	551	577	595	618	684	742	798
Mean	45	82	126	163	213	252	291	328	361	394	445	490	532	563	585	610	677	738	794
Exposed Web W6	102	176	255	329	397	431	468	505	538	570	616	652	689	712	725	748	822	878	921
W7	91	172	249	320	389	429	469	504	533	561	604	644	681	700	716	738	813	869	912
Mean	97	174	252	325	393	430	469	505	536	566	610	648	685	706	721	743	818	874	917
Lower Flange F6	113	210	316	413	493	540	582	618	651	678	718	751	782	799	808	828	891	941	978
F7	99	205	317	417	491	533	573	606	637	665	707	740	771	790	799	818	887	933	968
Mean	106	208	317	415	492	537	578	612	644	672	713	746	777	795	804	823	889	937	973
<b>Beam @ 500 mm from End Plate</b>																			
Upper Flange F1	13	14	15	20	25	32	42	52	64	76	90	101	108	116	125	134	153	179	209
F4	11	12	14	19	25	31	40	50	60	71	85	96	107	117	128	136	156	181	208
Mean	12	13	15	20	25	32	41	51	62	74	88	99	108	117	127	135	155	180	209
Unexposed Web W1	15	22	34	51	70	87	104	123	141	157	183	207	231	250	268	284	319	356	391
W4	12	19	29	43	61	76	96	107	117	137	162	183	206	225	241	257	289	325	361
Mean	14	21	32	47	66	82	100	115	129	147	173	195	219	238	255	271	304	341	376
Unexposed Angle A1	21	42	69	103	117	168	201	233	265	292	335	375	411	442	467	488	543	593	639
A4	18	38	65	98	122	162	190	220	251	277	319	359	392	423	447	466	520	573	620
Mean	20	40	67	101	120	165	196	227	258	285	327	367	402	433	457	477	532	583	630
Exposed Angle A5	50	85	154	218	282	332	379	424	464	501	557	604	651	680	697	721	791	862	913
A8	55	106	164	223	292	334	380	421	460	495	545	592	631	656	672	693	762	829	887
Mean	53	96	159	221	287	333	380	423	462	498	551	598	641	668	685	707	777	846	900
Exposed Web W5	148	281	415	523	599	632	658	686	710	732	756	787	816	822	825	848	913	953	987
W8	139	258	374	475	549	589	624	653	679	701	733	761	792	795	799	819	894	936	971
Mean	144	270	395	499	574	611	641	670	695	717	745	774	804	809	812	834	904	945	979
Lower Flange F5	141	285	433	557	635	673	699	722	740	751	777	806	834	846	848	871	930	967	998
F8	135	268	416	531	607	642	671	694	714	732	751	784	812	817	818	838	905	944	976
Mean	138	277	425	544	621	658	685	708	727	742	764	795	823	832	833	855	918	956	987
<b>Column/ End Plate Connection</b>																			
Plate/ Flange P1	18	32	64	110	138	168	205	245	285	323	378	436	486	533	565	592	655	713	769
P2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Mean	18	32	64	110	138	168	205	245	285	323	378	436	486	533	565	592	655	713	769
Web W9	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
<b>Column @ 300 mm from Base</b>																			
Flanges F9	67	142	239	334	419	473	526	571	608	643	691	725	762	787	799	817	893	939	974
F10	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Mean	67	142	239	334	419	473	526	571	608	643	691	725	762	787	799	817	893	939	944
Web W10	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
<b>Unexposed Bolts</b>																			
B1	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
B2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Mean	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
<b>Exposed Bolts</b>																			
B3	37	64	115	173	232	280	322	363	400	436	492	546	597	626	652	687	747	811	876
B4	36	65	117	172	227	268	316	363	406	447	508	560	604	638	660	683	743	810	872
Mean	37	65	116	173	230	274	319	363	403	442	500	553	601	632	656	685	745	811	874
Mean Furnace Gas Standard Curve	504	601	663	699	726	741	757	773	786	802	824	847	867	851	859	895	945	977	1006
	502	603	663	705	739	766	789	809	826	842	865	885	902	918	932	945	968	988	1006
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																			
Transducers T1	*	-0.1	*	2.3	*	1.6	*	0.9	*	1.6	*	6.5	*	18.5	*	28.3	55.8	58.5	66.7
T2	*	-3.7	*	-6.3	*	-7.8	*	-6.0	*	-2.6	*	4.1	10.5	19.8	26.3	33.6	92.4	96.4	112
Mean	*	-1.9	*	-2.0	*	-3.1	*	-2.6	*	-0.5	*	5.3	*	19.2	*	31.0	74.1	77.5	89.4
Tilt Sensor	*	-2.6	*	-5.2	*	-4.6	*	-2.1	*	1.0	*	*	11.9	19.6	22.7	30.5	73.9	78.5	90.9

DATA  
SHEET  
NUMBER

119C

SHELF ANGLE FLOOR BEAM / COLUMN (MAJOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**120A****BEAM / BEAM (MINOR AXIS) CONNECTION****DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
406 × 178 Beam	Nominal Actual	60	406.4	177.8	7.8	12.8	1058	134.8	1194	208.3	21508	1199
305 × 165 Beam/Column	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
406 UB	Grade 43A													
305 UB	Grade 43A	0.17	0.23	0.64	0.015	0.017	0.03	0.005	0.04	<0.005	0.03	<0.005	0.035	0.0065

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 27th February 1990  
 TEST NUMBER : WFRC 46733

## ASSESSMENT

LOAD BEARING CAPACITY  
 46 minutes at 0.2 M<sub>p</sub>

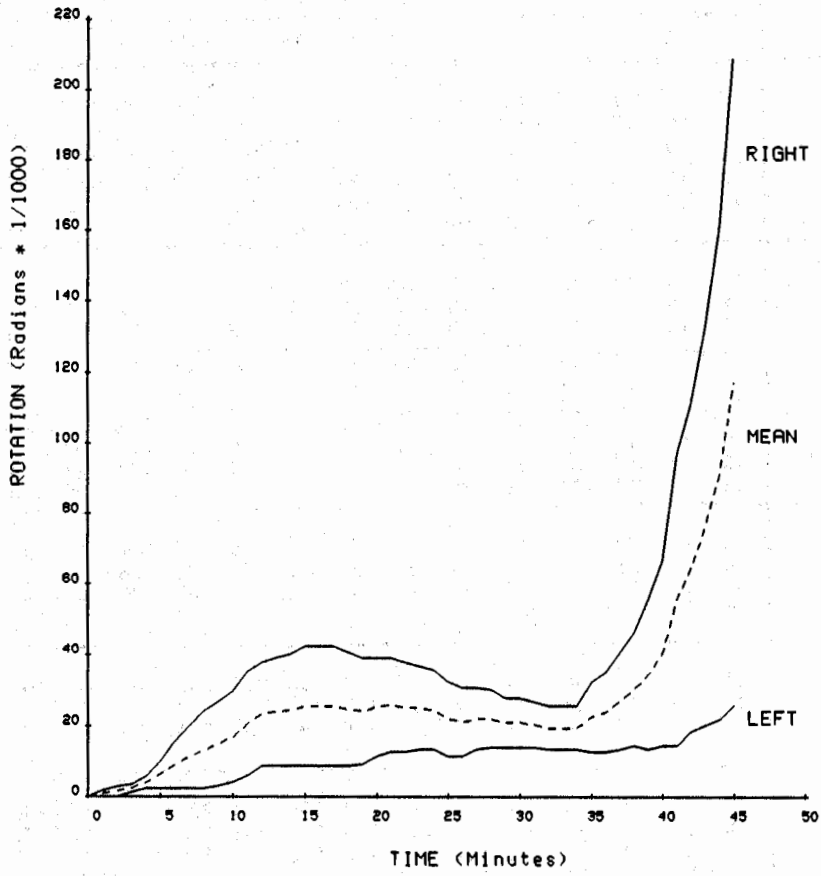
DATA SHEET NUMBER **120B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																	
	2	3	4	6	8	9	10	12	15	18	21	24	27	30	35	40	45	46
<b>Beam @ 25 mm from End Plate</b>																		
Web W2	30	35	41	59	81	92	105	128	169	211	256	302	348	392	462	528	587	598
W3	19	23	29	43	61	71	86	110	152	195	241	287	333	379	450	516	575	585
Mean	25	29	35	51	71	82	96	119	161	203	249	295	341	386	456	522	581	592
Lower Flange F6	15	19	23	35	50	59	69	88	126	169	214	259	305	352	424	492	554	567
F7	17	22	29	48	70	83	99	125	167	215	266	313	363	411	483	549	607	618
Mean	16	21	26	42	60	71	84	107	147	192	240	286	334	382	454	521	581	593
<b>Beam @ 500 mm from End Plate</b>																		
Upper Flange F1	13	13	13	15	17	18	19	22	27	34	41	47	54	62	75	87	102	104
F4	12	12	12	14	16	18	19	22	29	37	48	58	70	84	101	110	116	118
Mean	13	13	13	15	17	18	19	22	28	36	45	53	62	73	88	99	109	111
Web W1	12	12	14	18	25	29	35	46	64	80	98	119	140	162	198	236	273	282
W4	12	13	14	18	23	27	31	40	55	71	85	103	124	145	179	215	250	258
Mean	12	13	14	18	24	28	33	43	60	76	92	111	132	154	189	226	262	270
Lower Flange F5	14	15	17	20	25	28	32	40	58	75	94	113	135	160	202	246	293	302
F8	13	14	16	20	26	28	31	39	53	69	85	102	122	145	171	212	256	265
Mean	14	15	17	20	26	28	32	40	56	72	90	108	129	153	187	229	275	284
<b>Cross Beam</b>																		
Upper Flange F2	14	17	22	32	45	52	59	72	93	118	152	184	215	248	297	343	389	413
F3	20	25	31	45	59	66	74	90	118	144	178	208	240	271	316	365	427	445
Mean	17	21	27	39	52	59	67	81	106	131	165	196	228	260	307	354	408	429
Lower Flange F9	55	83	117	197	280	325	376	450	546	619	671	711	740	769	818	855	890	895
<b>Cross Beam / End Plate Connection</b>																		
Plate / Web P1	22	29	38	58	83	97	115	144	195	248	301	354	406	460	539	608	666	679
P2	21	29	38	58	83	98	117	147	198	252	307	361	414	466	543	610	669	680
Mean	22	29	38	58	83	98	116	146	197	250	304	358	410	463	541	609	668	680
<b>Exposed Bolts</b>																		
Upper B1	23	30	38	56	79	93	114	140	188	238	290	339	388	438	512	577	632	643
B2	24	34	43	65	93	109	128	160	211	264	319	371	422	473	547	611	665	676
Mean	24	32	41	61	86	101	121	150	200	251	305	355	405	456	530	594	649	660
Lower B3	22	29	37	52	76	92	112	145	198	253	309	361	414	466	540	606	658	667
B4	23	30	38	57	85	103	124	156	210	267	325	380	434	487	563	624	678	689
Mean	23	30	38	55	81	98	118	151	204	260	317	371	424	477	552	615	668	678
Mean Furnace Gas Standard Curve	448	496	542	605	643	671	685	716	738	769	793	810	833	844	878	896	913	919
	445	502	544	603	645	663	678	705	739	766	789	809	826	842	865	885	902	906
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																		
Transducers T1	0	1.2	2.3	2.3	2.3	2.9	4.0	8.7	8.7	8.7	12.7	13.3	13.3	13.9	12.7	14.4	26.0	*
T2	2.8	3.3	5.6	15.6	24.0	26.8	29.6	37.9	42.4	40.7	39.1	35.7	30.7	27.9	32.4	66.9	209	*
Mean	1.4	2.3	4.0	9.0	13.2	14.9	16.8	23.3	25.6	24.7	25.9	24.5	22.0	20.9	22.6	40.7	118	*



DATA SHEET NUMBER **120C**

BEAM / BEAM (MINOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**121A****BEAM / BEAM (MINOR AXIS) CONNECTION****DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
406 × 178 Beam	Nominal Actual	60	406.4	177.8	7.8	12.8	1058	134.8	1194	208.3	21508	1199
305 × 165 Beam/Column	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
406 UB 305 UB	Grade 43A Grade 43A	0.17	0.23	0.64	0.015	0.017	0.03	0.005	0.04	<0.005	0.03	<0.005	0.035	0.0065

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 27th February 1990  
 TEST NUMBER : WFRS 46733

## ASSESSMENT

LOAD BEARING CAPACITY  
 47 minutes at 0.4 M<sub>p</sub>

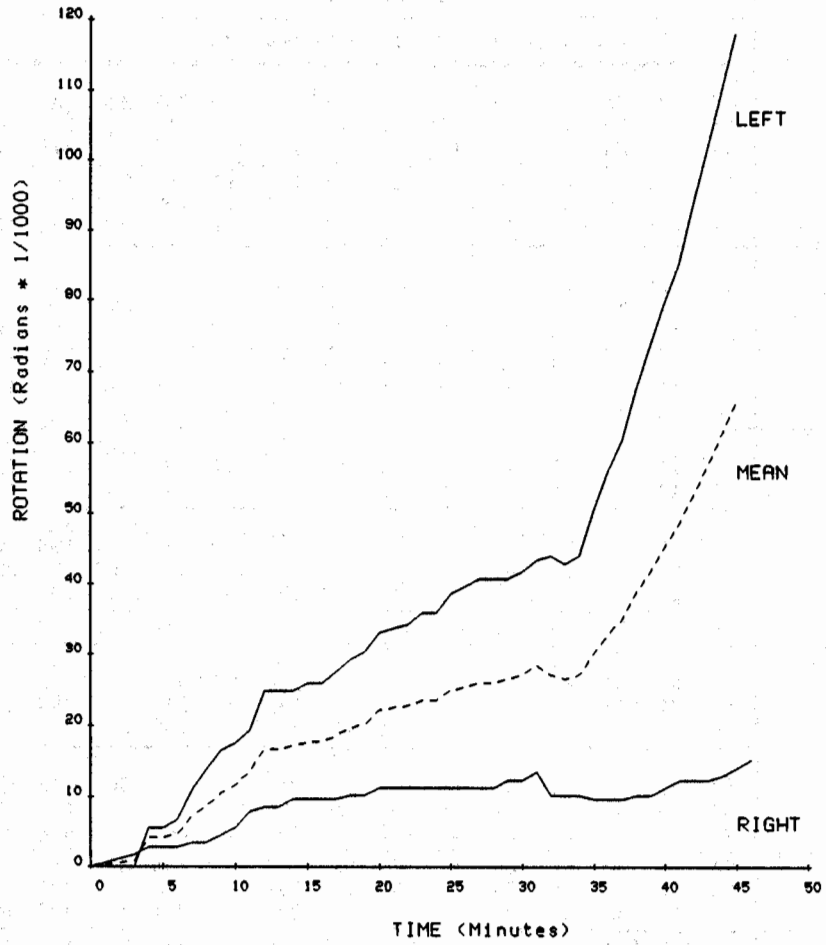
DATA SHEET NUMBER **121B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																	
	2	3	4	6	8	9	10	12	15	18	21	24	27	30	35	40	45	47
<b>Beam @ 25 mm from End Plate</b>																		
Web																		
W2	18	22	28	42	59	68	80	105	149	195	242	288	334	381	451	517	575	596
W3	16	20	24	37	52	60	71	91	130	172	216	261	307	354	424	491	551	573
Mean	17	21	26	40	56	64	76	98	140	184	229	275	321	368	438	504	563	585
Lower Flange																		
F6	17	21	26	40	58	69	82	106	156	207	259	310	360	410	482	549	607	628
F7	14	16	19	29	42	49	59	77	115	161	207	255	302	351	423	492	554	576
Mean	16	19	23	35	50	59	71	92	136	184	233	283	331	381	453	521	581	602
<b>Beam @ 500 mm from End Plate</b>																		
Upper Flange																		
F1	10	10	10	11	13	15	17	22	30	42	54	65	77	88	111	137	165	176
F4	11	11	11	13	16	18	21	26	37	49	61	73	85	99	123	154	187	199
Mean	11	11	11	12	15	17	19	24	34	46	58	69	81	94	117	146	176	188
Web																		
W1	13	13	15	18	23	26	31	39	56	73	90	109	131	154	191	231	272	289
W4	13	14	16	20	26	29	34	42	58	75	93	113	135	158	195	234	274	291
Mean	13	14	16	19	25	28	33	41	57	74	92	111	133	156	193	233	273	290
Lower Flange																		
F5	11	12	12	14	17	19	22	30	46	63	75	91	113	138	179	222	268	286
F8	11	11	12	13	15	16	19	24	35	50	67	83	103	127	166	207	251	269
Mean	11	12	12	14	16	18	21	27	41	57	71	87	108	133	173	215	260	278
<b>Cross Beam</b>																		
Upper Flange																		
F2	23	26	29	39	50	56	62	74	95	120	145	171	198	227	276	328	375	396
F3	18	21	25	33	42	47	54	66	87	113	135	168	200	231	278	324	368	385
Mean	21	24	27	36	46	52	58	70	91	117	140	170	199	229	277	326	372	391
Lower Flange																		
F9	69	96	126	198	290	336	387	462	559	632	681	720	747	777	823	858	889	901
<b>Cross Beam / End Plate Connection</b>																		
Plate / Web																		
P1	27	34	42	62	87	102	119	151	206	263	317	373	426	478	550	614	668	689
P2	21	27	34	50	69	81	96	123	172	225	279	334	388	441	519	586	644	664
Mean	24	31	38	56	78	92	108	137	189	244	298	354	407	460	535	600	656	677
<b>Exposed Bolts</b>																		
Upper																		
B1	24	32	41	63	91	108	127	159	210	264	317	368	419	469	543	607	660	680
B2	23	30	37	54	77	90	106	136	188	242	294	346	398	450	522	586	639	659
Mean	24	31	39	59	84	99	117	148	199	253	306	357	409	460	533	597	650	670
Lower																		
B3	24	32	40	61	89	107	128	163	220	277	335	390	443	495	568	628	680	698
B4	23	29	37	54	76	90	107	140	197	255	313	370	425	478	552	616	670	690
Mean	24	31	39	58	83	99	118	152	209	266	324	380	434	487	560	622	675	694
Mean Furnace Gas																		
Standard Curve	448	496	542	605	643	671	685	716	738	769	793	810	833	844	878	896	913	926
	445	502	544	603	645	663	678	705	739	766	789	809	826	842	865	885	902	909
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																		
Transducers																		
T1	0	0	5.5	6.6	13.8	16.6	17.7	24.8	25.9	29.3	33.7	35.9	40.8	41.9	50.8	79.9	118	*
T2	1.1	1.7	2.8	2.8	3.4	4.5	5.6	8.5	9.6	10.2	11.3	11.3	11.3	12.4	9.6	11.3	14.1	*
Mean	0.6	0.9	4.2	4.7	8.6	10.6	11.7	16.7	17.8	19.8	22.5	23.6	26.1	27.2	30.2	45.6	66.1	*

DATA  
SHEET  
NUMBER

121C

BEAM / BEAM (MINOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**122A**

**BEAM / BEAM (MINOR AXIS) CONNECTION**

**DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
406 × 178 Beam	Nominal Actual	60	406.4	177.8	7.8	12.8	1058	134.8	1194	208.3	21508	1199
305 × 165 Beam/Column	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
406 UB 305 UB	Grade 43A Grade 43A	0.17	0.23	0.64	0.015	0.017	0.03	0.005	0.04	<0.005	0.03	<0.005	0.035	0.0065

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 1st March 1990  
 TEST NUMBER : WFRS 46736

## ASSESSMENT

LOAD BEARING CAPACITY  
 30 minutes at 0.1 M<sub>p</sub>

DATA SHEET NUMBER **122B**

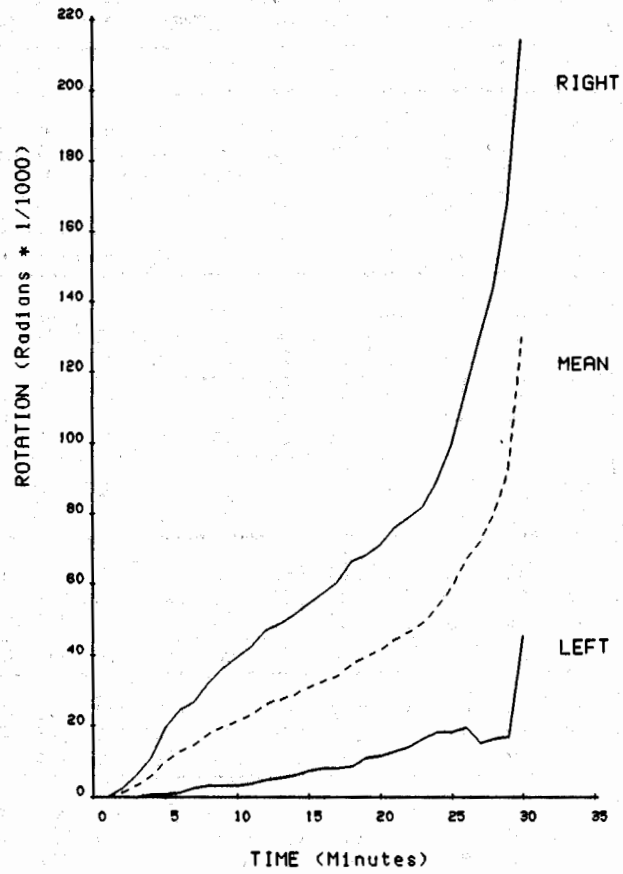
THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)															
	2	3	4	6	8	9	10	12	15	18	21	24	27	28	29	30
<b>Beam @ 25 mm from End</b>																
Web																
W2	32	43	56	82	111	128	145	182	235	295	351	409	461	479	496	512
W3	32	42	53	74	100	118	134	169	220	276	333	390	443	460	477	494
Mean	32	43	55	78	106	123	140	176	228	286	342	400	452	470	487	503
<b>Lower Flange</b>																
F6	14	15	18	29	42	49	57	72	98	134	174	218	265	281	297	312
F7	15	17	20	32	45	52	58	73	99	138	178	225	272	288	304	320
Mean	15	16	19	31	44	51	58	73	99	136	176	222	269	285	301	316
<b>Beam @ 500 mm from End</b>																
<b>Upper Flange</b>																
F1	13	13	14	16	19	21	22	27	34	42	50	57	64	67	69	73
F4	12	13	15	18	23	26	31	39	51	63	75	87	96	98	100	101
Mean	13	13	15	17	21	24	27	33	43	53	63	72	80	83	85	87
<b>Web</b>																
W1	13	15	17	24	32	36	41	52	67	83	103	124	145	152	159	166
W4	14	16	18	27	36	42	48	62	79	101	124	148	172	180	188	196
Mean	14	16	18	26	34	39	45	57	73	92	114	136	159	166	174	181
<b>Lower Flange</b>																
F5	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
F8	12	12	13	17	24	28	32	44	61	78	97	122	148	157	166	175
Mean	12	12	13	17	24	28	32	44	61	78	97	122	148	157	166	175
<b>Cross Beam</b>																
<b>Upper Flange</b>																
F2	20	32	76	124	145	158	173	201	237	280	316	358	396	408	419	431
F3	49	60	74	99	126	145	157	188	229	271	318	360	397	410	426	442
Mean	35	46	75	112	136	152	165	195	233	276	317	359	397	409	423	437
<b>Lower Flange</b>																
F9	79	126	167	244	320	364	406	487	574	648	699	737	760	770	780	790
<b>Cross Beam / Web Cleat Connection</b>																
<b>Cleat / Web</b>																
C1	33	48	63	97	132	152	172	217	278	344	406	465	517	534	550	565
C2	36	49	62	95	130	153	174	218	280	344	407	466	518	534	550	565
Mean	35	49	63	96	131	153	173	218	279	344	407	466	518	534	550	565
<b>Bolts (Cleats / Test Beams)</b>																
<b>Upper</b>																
B1	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
B2	30	45	62	97	132	151	171	213	270	331	388	444	493	509	524	540
Mean	30	45	62	97	132	151	171	213	270	331	388	444	493	509	524	540
<b>Lower</b>																
B3	29	40	50	79	110	128	148	190	250	315	376	435	487	503	519	534
B4	33	46	59	90	125	145	166	212	275	342	404	463	514	531	546	562
Mean	31	43	55	85	118	137	157	201	263	329	390	449	501	517	533	548
<b>Bolts (Cleats / Cross Beam)</b>																
<b>Upper</b>																
B5	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
B6	29	40	50	74	102	119	136	172	222	277	333	385	436	453	469	486
Mean	29	40	50	74	102	119	136	172	222	277	333	385	436	453	469	486
<b>Lower</b>																
B7	26	35	44	65	89	104	120	156	206	264	320	376	426	444	460	476
B8	32	42	51	71	93	110	126	160	209	263	321	379	432	449	467	484
Mean	29	39	48	68	91	107	123	158	208	264	321	378	429	447	464	480
<b>Mean Furnace Gas Standard Curve</b>																
	529	562	581	603	639	666	688	719	739	763	804	815	829	831	846	852
	445	502	544	603	645	663	678	705	739	766	789	809	826	832	837	842
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																
<b>Transducers</b>																
T1	0	0	0.6	1.2	3.0	3.0	3.0	4.8	7.3	8.5	12.7	18.1	15.1	16.3	16.9	45.3
T2	2.4	6.0	10.9	24.2	32.0	36.3	39.3	47.1	54.4	66.4	76.1	89.3	129	144	168	214
Mean	1.2	3.0	5.8	12.7	17.5	19.7	21.2	26.0	30.9	37.5	44.4	53.7	72.1	80.2	92.5	129.7



DATA  
SHEET  
NUMBER

122C

BEAM / BEAM (MINOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER

**123A**

**BEAM / COLUMN (MINOR AXIS) CONNECTION**

**DIMENSIONS AND PROPERTIES**

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
254 × 254 Column	Nominal Actual	73	254.0	254.0	8.6	14.2	894.5	305.0	988.6	462.4	11360	3873
305 × 165 Beam	Nominal Actual	40	303.8	165.1	6.1	10.2	561.2	92.4	624.5	141.5	8523	763

**CHEMICAL COMPOSITION (PRODUCT ANALYSIS - Wt. %)**

Section	Steel Quality	C	Si	Mn	P	S	Cr	Mo	Ni	V	Cu	Nb	Al	N
Column	Grade 43A	0.13	0.22	0.59	0.037	0.028	0.12	0.028	0.16	<0.005	0.61	<0.005	0.012	0.012
Beam	Grade 43A	0.17	0.23	0.64	0.015	0.017	0.03	0.005	0.04	<0.005	0.03	<0.005	0.035	0.0065

**ROOM TEMPERATURE TENSILE PROPERTIES**

Position	LYS (N/mm <sup>2</sup> )	UTS (N/mm <sup>2</sup> )	Elongation (%)

**TEST CONDITIONS**

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**NOTES**

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TEST CENTRE : Warrington Research  
 TEST DATE : 1st March 1990  
 TEST NUMBER : WFRC 46736

## ASSESSMENT

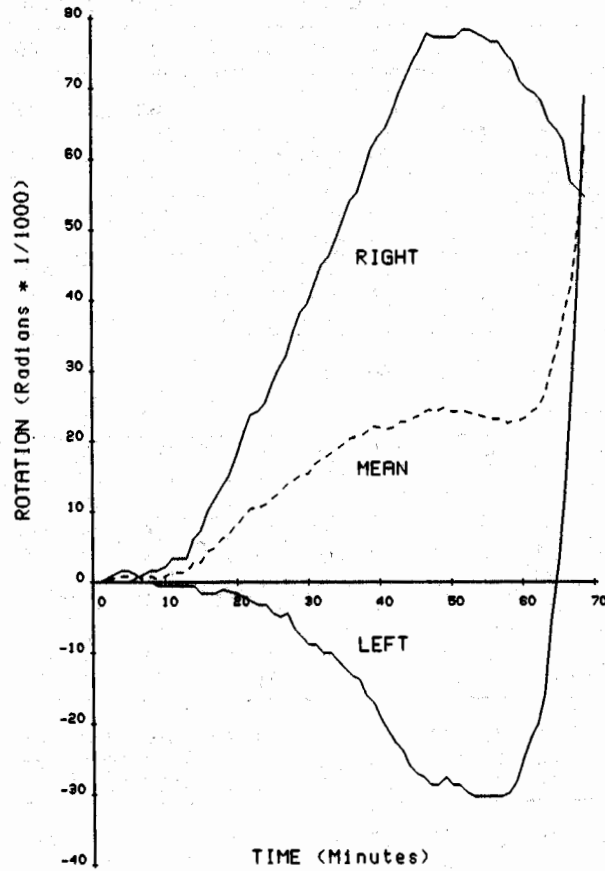
LOAD BEARING CAPACITY  
 69 minutes at 0.2 M<sub>p</sub>

DATA SHEET NUMBER **123B**

THERMOCOUPLE LOCATION	TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																	
	3	6	9	12	15	18	21	24	27	30	35	40	45	50	55	60	65	69
<b>Beam @ 25 mm from End Plate</b>																		
Upper Flange F2	28	43	58	73	86	107	127	149	171	194	231	267	300	334	368	400	431	455
F3	14	22	32	45	59	74	91	112	133	154	186	201	217	249	287	323	356	380
Mean	21	33	45	59	73	91	109	131	152	174	209	234	259	292	328	362	394	418
Web W2	20	38	55	76	101	131	162	197	232	268	330	390	446	498	546	590	632	663
W3	20	33	55	83	106	133	162	193	223	254	307	359	406	451	495	539	583	615
Mean	20	36	55	80	104	132	162	195	228	261	319	375	426	475	521	565	608	639
Lower Flange F6	23	43	63	91	121	156	194	234	272	311	375	436	494	547	596	640	680	708
F7	16	30	47	65	87	115	146	180	215	252	316	378	439	496	548	596	639	671
Mean	20	37	55	78	104	136	170	207	244	282	346	407	467	522	572	618	660	690
<b>Beam @ 500 mm from End Plate</b>																		
Upper Flange F1	12	16	19	26	33	40	48	57	65	73	87	100	109	118	131	150	169	183
F4	12	15	19	26	33	41	50	57	64	71	85	100	109	112	122	141	167	187
Mean	12	16	19	26	33	41	49	57	65	72	86	100	109	115	127	146	168	185
Web W1	16	28	44	65	83	103	125	150	173	197	237	278	318	358	396	435	473	503
W4	15	25	37	55	71	90	113	137	161	184	222	260	297	334	370	407	446	476
Mean	16	27	41	60	77	97	119	144	167	191	230	269	308	346	383	421	460	490
Lower Flange F5	13	18	28	44	61	79	98	124	149	175	222	270	319	369	418	466	513	549
F8	13	20	31	49	70	93	114	138	163	189	233	279	326	373	419	465	510	544
Mean	13	19	30	47	66	86	106	131	156	182	228	275	323	371	419	466	512	547
<b>Column / End Plate Connection</b>																		
Plate / Web P1	72	95	130	158	186	224	271	313	357	402	468	530	583	632	673	711	745	763
P2	33	51	74	110	144	183	226	273	319	367	442	509	566	618	662	701	736	753
Mean	53	73	102	134	165	204	249	293	338	385	455	520	575	625	668	706	741	758
<b>Column Flanges</b>																		
F9	100	176	255	341	410	479	541	589	626	660	706	738	767	798	823	848	871	888
F10	121	175	243	314	382	450	513	566	606	643	692	731	758	790	818	843	868	884
Mean	111	176	249	328	396	465	527	578	616	652	699	735	763	794	821	846	870	886
<b>Exposed Bolts</b>																		
Upper B1	30	48	70	99	129	162	197	232	267	303	362	416	465	509	552	591	627	655
B2	37	55	75	104	133	167	*	*	*	*	*	*	*	*	*	*	*	*
Mean	34	52	73	102	131	165	197	232	267	303	362	416	465	509	552	591	627	655
Lower B3	26	46	71	109	147	192	239	289	338	388	467	537	600	652	697	734	760	784
B4	33	51	72	107	144	187	231	280	329	378	455	525	586	638	682	721	750	768
Mean	30	49	72	108	146	190	235	285	334	383	461	531	593	645	690	728	755	776
<b>Mean Furnace Gas Standard Curve</b>																		
	562	603	666	719	739	763	804	815	829	852	868	892	906	915	934	959	971	979
	502	603	663	705	739	766	789	809	826	842	865	885	902	918	932	945	957	966
<b>Rotation (Radians x 10<sup>-3</sup>)</b>																		
Transducers T1	1.1	1.1	-0.6	-0.6	-1.7	-1.1	-2.2	-3.3	-4.4	-8.8	-12.1	-19.3	-27.0	-28.7	-30.4	-24.8	2.8	68.9
T2	0.0	0.6	1.7	3.4	7.3	13.5	20.9	25.4	32.2	39.5	51.3	63.1	73.8	77.2	77.2	71.0	64.3	54.7
Mean	0.6	0.9	0.6	1.4	2.8	6.2	9.4	11.1	13.9	15.4	19.6	21.9	23.4	24.3	23.4	23.1	33.6	61.8

DATA SHEET NUMBER **123C**

BEAM / COLUMN (MINOR AXIS) CONNECTION





DATA  
SHEET  
NUMBER **124**

INDICATIVE COLUMN WITH BLOCKED-IN WEB

FURNACE TYPE: FLOOR FURNACE  
POSITION IN FURNACE: FREE STANDING ON PLINTH

TEST CENTRE : Warrington Research  
TEST DATE : 25th January 1989  
TEST NUMBER : WFRS 44992

## DIMENSIONS AND PROPERTIES

Section Serial Size and Type (mm)	Dimensions and Properties	Mass per Metre (kg)	Depth of Section (mm)	Width of Section (mm)	Thickness		Elastic Modulus		Plastic Modulus		Moment of Inertia	
					Web (mm)	Flange (mm)	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>3</sup> )	Axis y-y (cm <sup>3</sup> )	Axis x-x (cm <sup>4</sup> )	Axis y-y (cm <sup>4</sup> )
356 × 406 Column	Nominal Actual	634	474.7	424.1	47.6	77.0	11592	4632	14247	7114	275140	98211

## THERMAL DATA

THERMOCOUPLE LOCATION		TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																		
		6	12	18	24	27	30	35	40	45	50	55	60	65	70	75	80	85	90	98
Flanges	F1	32	88	157	226	260	294	349	404	457	506	550	588	624	659	690	717	740	762	813
	F2	36	106	183	247	281	316	376	439	499	553	596	631	669	701	729	751	780	808	856
	Mean	34	97	170	237	271	305	363	422	478	530	573	610	647	680	710	734	760	785	835
Flange / Web Junction	F3	24	64	119	174	202	230	275	322	370	415	458	496	531	567	601	631	656	680	715
	F4	27	69	126	175	201	228	276	327	378	423	463	499	536	572	603	629	652	672	705
	Mean	26	67	123	175	202	229	276	325	374	419	461	498	534	570	602	630	654	676	710
Web	W1	14	23	42	73	90	109	137	168	200	231	262	292	322	350	378	404	430	454	491
	W3	14	24	47	82	99	110	140	171	204	234	265	295	325	355	382	410	435	458	495
	Mean	14	24	45	78	95	110	139	170	202	233	264	294	324	353	380	407	433	456	493
Web	W2	10	13	25	48	62	77	103	133	161	189	217	246	274	301	327	354	379	404	443
Mean Furnace Gas Standard Curve		601	699	741	773	786	802	824	847	867	851	859	895	925	945	961	977	992	1006	1027
		603	705	766	809	826	842	865	885	902	918	932	945	957	968	979	988	997	1006	1019

## NOTES

**APPENDIX 2****THERMAL DATA ASSOCIATED WITH THE CONCRETE FLOOR SLABS**

As noted in Section 3.1.4.1 thermocouples were installed at various positions within two of the concrete floor slabs in order to monitor the temperatures of the concrete, shear connectors, metal decking and mesh reinforcement. The assemblies involved were Nos. 6 and 7. Unfortunately this particular aspect of the data recording exercise was not entirely successful and consequently it did not yield as much information as was hoped for. An unusually high number of thermocouple failures were experienced, especially on Assembly No. 6. At the time, these were attributed to problems caused by either the presence of free water in the concrete, causing the thermocouples to short circuit, or mechanical damage to the wiring resulting from flexing of the floor slabs.

For these reasons the data associated with the concrete floor slabs are being presented separately.

**A2.1 ASSEMBLY NO. 6**

Thermocouples were installed at 18 positions as indicated in Fig. A2.1. These may be summarised as:

- Six in the concrete at a trough position in the metal decking to give a vertical thermal profile at 20 mm increments, (T Series).
- Three in the concrete at a crest position in the metal decking to give a vertical thermal profile at 20 mm increments, (R Series).
- One in the concrete at the edge of the metal decking, (Coded RE).
- Four on shear connectors, (Coded S).
- Four on the reinforcing mesh, (Coded RF).

Of the 18 thermocouples installed only 8 are considered to have generated reliable data and these are presented in Data Sheet No. 117D.

**A2.2 ASSEMBLY NO. 7**

Thermocouples were installed at the same 18 positions as described for Assembly No. 6. Only 11 are considered to have generated reliable data and these are presented in Data Sheet No. 118D.



TEST CENTRE : Warrington Research  
 TEST DATE : 11th January 1989  
 TEST NUMBER : WFRC 44519

DATA SHEET NUMBER **117D**

THERMOCOUPLE LOCATION		TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																			
		2	6	12	18	24	28	30	36	40	46	50	56	60	70	80	90	100	110	123	
<u>Concrete</u>																					
T Series	T	77	183	352	484	581	645	670	730	759	809	840	*	*	*	*	*	*	*	*	
	T20	21	77	116	178	246	297	320	389	427	483	516	560	587	652	*	*	*	*	*	
	T40	12	32	107	117	124	147	170	233	272	323	353	389	411	466	521	572	640	690	742	
	T60	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	
	T80	10	10	103	103	103	103	103	104	108	110	112	125	138	171	211	249	296	331	390	
	T100	10	14	79	109	115	119	119	119	120	123	125	128	130	145	186	229	273	319	378	
R Series	R	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	
	R20	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	
	R40	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	
Deck Edge	RE	*	62	106	139	185	220	237	292	329	385	421	476	510	589	658	716	758	796	840	
<u>Shear Connectors</u>	S1	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	
	S1A	10	11	22	94	101	102	101	102	113	133	144	162	173	201	230	262	296	329	379	
	S2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	
	S2A	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
<u>Reinforcement</u>	RF1	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	
	RF1A	10	20	25	36	52	60	62	73	82	92	99	110	117	118	120	122	128	140	166	
	RF2	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
	RF2A	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*

TEST CENTRE : Warrington Research  
 TEST DATE : 18th January 1989  
 TEST NUMBER : WFRS 44175

DATA  
 SHEET NUMBER **118D**

THERMOCOUPLE LOCATION		TEMPERATURE Deg. C AFTER VARIOUS TIMES (MINUTES)																	
		2	6	12	18	24	27	30	36	40	46	50	56	60	70	80	90	100	111
<u>Concrete</u>																			
T Series	T	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
	T20	17	90	105	103	147	200	239	317	365	426	461	508	537	598	652	703	753	838
	T40	14	34	105	104	106	110	120	171	203	252	284	330	357	418	475	532	590	777
	T60	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
	T80	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
	T100	14	15	103	105	106	106	106	106	106	106	105	105	110	131	164	205	264	438
R Series	R	16	58	86	95	101	106	123	166	185	209	221	242	255	292	328	367	414	512
	R20	13	17	35	51	85	94	94	93	93	94	99	103	103	123	147	179	218	275
	R40	14	14	23	34	58	71	77	85	88	90	92	95	96	101	106	116	144	184
Deck Edge	RE	*	*	102	111	122	129	141	175	192	215	229	251	264	299	333	367	399	481
<u>Shear Connectors</u>	S1	14	15	38	103	103	103	103	104	107	118	125	138	147	174	206	238	273	314
	S1A	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
	S2	14	15	*	49	76	85	92	105	106	107	121	136	155	187	221	254	286	318
	S2A	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
<u>Reinforcement</u>	RF1	13	13	13	23	46	59	67	79	83	88	91	94	96	98	102	104	105	110
	RF1A	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
	RF2	14	14	37	69	85	90	93	98	102	105	104	105	105	117	148	180	215	263
	RF2A	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*

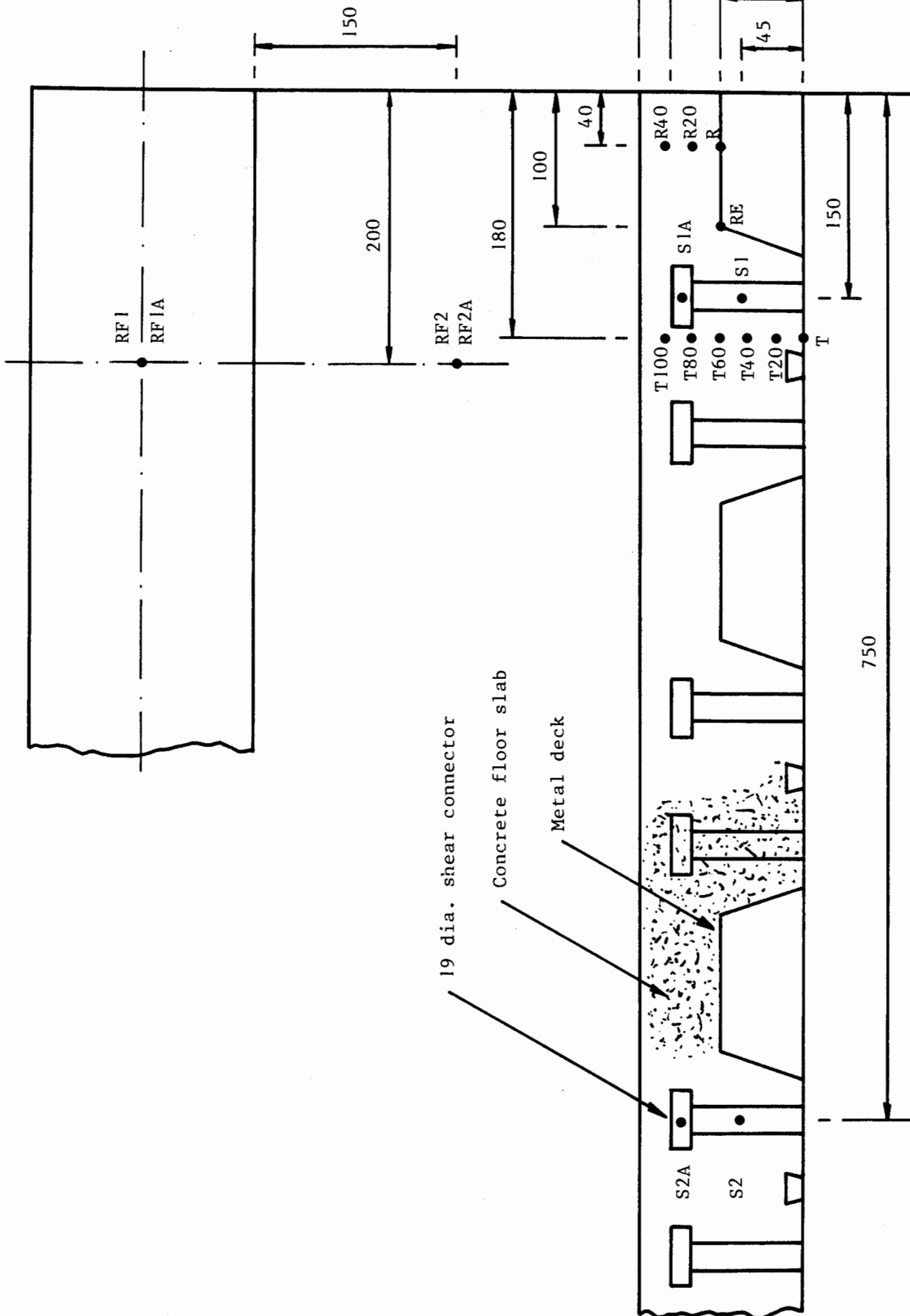


FIG. A2.1

CONCRETE, SHEAR STUD AND REINFORCEMENT THERMOCOUPLE POSITIONS

(R4/3668A)

## APPENDIX 3

## SUMMARY OF LOADS ON JACKS (TEST ASSEMBLIES 1-8)

## A3.1 NON-COMPOSITE BEAM ASSEMBLIES

$$\begin{aligned} \text{Floor slab width} &= 620 \text{ mm} \\ \text{Floor slab depth} &= 120 \text{ mm} \end{aligned}$$

Density of normal weight concrete assumed to be  $2400 \text{ kg/m}^3$ .

$$\begin{aligned} \text{Self weight of floor slab} &= 0.12 \times 0.62 \times 2400 \text{ kg/m} \\ &= \underline{178.56 \text{ kg/m}} \end{aligned}$$

$$\text{Self weight of steel beam} = \underline{40 \text{ kg/m}}$$

$$\begin{aligned} \text{Total self weight of system} &= 178.56 + 40 \text{ kg/m} \\ &= 218.56 \text{ kg/m} \\ &= \underline{2.143 \text{ kN/m}} \end{aligned}$$

$$\text{Beam length} = 1280 \text{ mm}$$

$$\begin{aligned} \text{Moment due to self weight} &= 2.143 \times (1.28)^2 / 2 \text{ kN m} \\ &= \underline{1.756 \text{ kN m}} \end{aligned}$$

## A3.1.1 Assembly No. 1

$$\text{Applied moment required} = 0.4 M_p$$

where  $M_p$  is the plastic moment capacity of the steel beam =  $171.6 \text{ kN m}$  (assuming specified grade of steel).

$$\begin{aligned} \text{Test moment} &= 0.4 \times 171.6 \text{ kN m} \\ &= \underline{68.64 \text{ kN m}} \end{aligned}$$

$$\begin{aligned} P \times \ell_a &= 68.64 - 1.756 \text{ kN m} \\ &= \underline{66.884 \text{ kN m}} \end{aligned}$$

$$\text{Lever arm, } \ell_a, = 980 \text{ mm}$$

$$\begin{aligned} \therefore P &= 66.884 / 0.98 \text{ kN} \\ &= 68.249 \text{ kN} \\ &= \underline{6.96 \text{ tonnes}} \end{aligned}$$

After 45 minutes the load was reduced to  $0.3 M_p$ .

$$\begin{aligned} \text{Test moment} &= 0.3 \times 171.6 \text{ kN m} \\ &= \underline{51.48 \text{ kN m}} \end{aligned}$$

$$\begin{aligned} P \times \ell_a &= 51.48 - 1.756 \text{ kN m} \\ &= \underline{49.724 \text{ kN m}} \end{aligned}$$

The lever arm remained at 980 mm.

$$\begin{aligned} \therefore P &= 49.724 / 0.98 \text{ kN} \\ &= 50.739 \text{ kN} \\ &= \underline{5.17 \text{ tonnes}} \end{aligned}$$

### Notes

#### Initial Loading

Test load applied was 6.9 tonnes  
Lever arm was 980 mm  
Actual test moment = 0.397  $M_p$

#### Reduced Loading

Test load applied was 5.1 tonnes  
Lever arm was 980 mm  
Actual test moment = 0.296  $M_p$

### **A3.1.2 Assembly No. 2**

$$\begin{aligned} \text{Applied moment required} &= 0.2 M_p \\ \text{Test moment} &= 0.2 \times 171.6 \text{ kN m} \\ &= \underline{34.32 \text{ kN m}} \\ P \times l_a &= 34.32 - 1.756 \text{ kN m} \\ &= \underline{32.564 \text{ kN m}} \\ \text{Lever arm, } l_a, &= 490 \text{ mm} \\ \therefore P &= 32.564 / 0.49 \text{ kN} \\ &= 66.457 \text{ kN} \\ &= \underline{6.78 \text{ tonnes}} \end{aligned}$$

### Note

Test load applied was 6.7 tonnes  
Lever arm was 490 mm  
Actual test moment = 0.198  $M_p$

### **A3.1.3 Assembly No. 3**

Moments and jack force calculations as for Assembly No. 2.

Test load applied was 6.7 tonnes  
Lever arm was 490 mm  
Actual test moment = 0.198  $M_p$

### **A3.1.4 Assembly No. 4**

Moments and jack force calculations as for Assembly No. 2.

Test load applied was 6.7 tonnes  
Lever arm was 490 mm  
Actual test moment = 0.198  $M_p$

**A3.1.5 Assembly No. 5**

Applied moment required	=	$0.1 M_p$
Test moment	=	$0.1 \times 171.6 \text{ kN m}$ = <u>17.16 kN m</u>
$P \times \ell_a$	=	$17.16 - 1.756 \text{ kN m}$ = <u>15.404 kN m</u>
Lever arm, $\ell_a$ ,	=	490 mm
$\therefore P$	=	$15.404 / 0.49 \text{ kN}$ = 31.437 kN = <u>3.21 tonnes</u>

Notes

Test load applied was 3.3 tonnes  
Lever arm was 490 mm  
Actual test moment =  $0.103 M_p$

**A3.2 COMPOSITE BEAM ASSEMBLIES**

Floor slab width	=	1000 mm
Average floor slab thickness	=	120 - 30 mm = 90 mm

Density of light-weight concrete assumed to be  $1200 \text{ kg/m}^3$ .

Self weight of floor slab	=	$0.09 \times 1.0 \times 1200 \text{ kg/m}$ = <u>108.0 kg/m</u>
Self weight of steel beam	=	<u>40 kg/m</u>
Self weight (steel + concrete)	=	$108.0 + 40 \text{ kg/m}$ = 148.0 kg/m = <u>1.451 kN/m</u>

Allow 0.14 kN/m for weight of steel decking.

Total self weight of system	=	<u>1.591 kN/m</u>
Moment due to self weight	=	$1.591 \times (1.28)^2 / 2 \text{ kN m}$ = <u>1.303 kN m</u>

**A3.2.1 Assembly No. 6**

Applied moment required	=	$0.4 M_p$
Test moment	=	$0.4 \times 171.6 \text{ kN m}$ = <u>68.64 kN m</u>
$P \times \ell_a$	=	$68.64 - 1.303 \text{ kN m}$ = <u>67.337 kN m</u>

$$\text{Lever arm, } \ell_a, \quad = \quad 980 \text{ mm}$$

$$\begin{aligned} \therefore P &= 67.337 / 0.98 \text{ kN} \\ &= 68.711 \text{ kN} \\ &= \underline{7.0 \text{ tonnes}} \end{aligned}$$

After 100 minutes the load was reduced to 0.3  $M_p$ .

$$\begin{aligned} \text{Test moment} &= 0.3 \times 171.6 \text{ kN m} \\ &= \underline{51.48 \text{ kN m}} \end{aligned}$$

$$\begin{aligned} P \times \ell_a &= 51.48 - 1.303 \text{ kN m} \\ &= \underline{50.177 \text{ kN m}} \end{aligned}$$

The lever arm remained at 980 mm.

$$\begin{aligned} \therefore P &= 50.177 / 0.98 \text{ kN} \\ &= 51.201 \text{ kN} \\ &= \underline{5.22 \text{ tonnes}} \end{aligned}$$

### Notes

#### Initial Loading

Test load applied was 6.7 tonnes  
Lever arm was 980 mm  
Actual test moment = 0.383  $M_p$

#### Reduced Loading

Test load applied was 5.0 tonnes  
Lever arm was 980 mm  
Actual test moment = 0.288  $M_p$

### **A3.2.2 Assembly No. 7**

$$\text{Applied moment required} \quad = \quad 0.2 M_p$$

$$\begin{aligned} \text{Test moment} &= 0.2 \times 171.6 \text{ kN m} \\ &= \underline{34.32 \text{ kN m}} \end{aligned}$$

$$\begin{aligned} P \times \ell_a &= 34.32 - 1.303 \text{ kN m} \\ &= \underline{33.017 \text{ kN m}} \end{aligned}$$

$$\text{Lever arm, } \ell_a, \quad = \quad 490 \text{ mm}$$

$$\begin{aligned} \therefore P &= 33.017 / 0.49 \text{ kN} \\ &= 67.382 \text{ kN} \\ &= \underline{6.87 \text{ tonnes}} \end{aligned}$$

### Note

Test load applied was 6.6 tonnes  
Lever arm was 490 mm  
Actual test moment = 0.192  $M_p$

**A3.2.3 Assembly No. 8**

Moments and jack force calculations as for Assembly No. 7.

After 70 minutes the load was reduced to  $0.1 M_p$ .

$$\begin{aligned} \text{Test moment} &= 0.1 \times 171.6 \text{ kN m} \\ &= \underline{17.16 \text{ kN m}} \end{aligned}$$

$$\begin{aligned} P \times \ell_a &= 17.16 - 1.303 \text{ kN m} \\ &= \underline{15.857 \text{ kN m}} \end{aligned}$$

The lever arm remained at 490 mm.

$$\begin{aligned} \therefore P &= 15.857 / 0.49 \text{ kN} \\ &= 32.361 \text{ kN} \\ &= \underline{3.3 \text{ tonnes}} \end{aligned}$$

**Notes****Initial Loading**

Test load applied was 6.6 tonnes

Lever arm was 490 mm

Actual test moment =  $0.192 M_p$

**Reduced Loading**

Test load applied was 3.0 tonnes

Lever arm was 490 mm

Actual test moment =  $0.092 M_p$



## APPENDIX 4

### PC DISK VERSION OF DATA

As mentioned in the Introduction to this report the data recorded during each of the twelve fire tests on connected assemblies are available on PC disks. The following section gives a brief outline of the material available and its format. The reader may find it useful to additionally consult reference 1.

The data are held on the disks in the form of ASCII text files. This format has been chosen since the majority of commercial software packages can import files of this type. The format allows the data to be referenced either via the screen, (or printer), or read directly by PC based software. The data are initially being made available on 3½ inch DSDD, 720 KB, floppy disks, but other disk sizes and formats can be supplied on request. The data files have been designated 'read only' in order to safeguard the user from accidentally corrupting or erasing them.

The data files are identified by reference to the DATA SHEET NUMBER sequence, i.e. from 112.DAT to 124.DAT inclusive. This numbering system is consistent with that introduced in reference 1. Thus, for example, data from test number WFRC 46733 can be found in data files 120.DAT and 121.DAT, (since two assemblies were tested simultaneously). For each individual assembly the thermal data have been subdivided into 'SETS' which reflect the thermocouple positions in the steelwork, and other materials. Mean temperature values are also included in these data sub-sets where it is considered valid to do so. In order that the columns of data in any particular 'SET' can be related to the corresponding thermocouple positions a 'README' file is associated with each data file. By way of example, README.120, which relates to data in file 120.DAT, is shown in Fig. A4.1.

It may be seen by reference to the data presented in Appendix 1 that there have been occasions when no temperature or deflection data were recorded. Such occurrences are indicated in the printed tables by the use of an asterisk. Since the use of such a character could cause problems if the software is expecting a numeric input, it has been replaced with the value zero in the disk held data files. It is obviously important for the user to ensure that any data have been read correctly by the particular software or program being used.

### REFERENCE

1. D.E. Wainman: 'Compendia of UK Standard Fire Test Data - Unprotected Structural Steel Nos. 1 and 2, PC Disk Version', Report SL/HED/R/S2298/1/92/C, British Steel Technical, Swinden Laboratories, 1992.

**TABLE A4.1**  
**README FILE ASSOCIATED WITH DATA FILE 120.DAT**

Data file 120.DAT contains data recorded during the ad-hoc fire resistance test number WFRS 46733 / ASSEMBLY No. 9 which is described in report number SL/HED/R/S2442/2/95/C - "SUMMARY OF DATA OBTAINED DURING TESTS ON BOLTED BEAM / COLUMN AND BEAM / BEAM CONNECTIONS" and should be read in conjunction with that document.

There are 29 items of data which, together with their mean values, are grouped in sets as shown below.

SET NUMBER	ITEMS IN COLUMNS
SET001.DAT	TIME, W2, W3, MEAN(2+3), F6, F7, MEAN(6+7)
SET002.DAT	TIME, F1, F4, MEAN(1+4), W1, W4, MEAN(1+4), F5, F8, MEAN(5+8)
SET003.DAT	TIME, F2, F3, MEAN(2+3), F9, P1, P2, MEAN(1+2)
SET004.DAT	TIME, B1, B2, MEAN(1+2), B3, B4, MEAN(3+4)
SET005.DAT	TIME, ISO, ATM1, ATM2, ATM3, ATM4, ATM5, ATM6, MEAN ATM
SET006.DAT	TIME, T1, T2, MEAN(1+2)